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HYDRAULIC AND OTHER TABLES

HYDRAULIC AND OTHER TABLES FOR PURPOSES OF SEWERAGE AND WATER-SUPPLY

BY

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SECOND EDITION, REVISED



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PREFACE TO SECOND EDITION.

THE First Edition of the Tables having become exhausted, the Author has thought it only right, before reprinting, to bring some parts of the work more nearly up to date.

For that purpose he has entirely rewritten Tables X., XI., XII. and XV., relating to Rainfall and Analysis of Water, availing himself for that purpose of more recent observations and researches; and the Introductory Remarks have been altered in accordance.

The subject of Flow in Pipes and Channels has been investigated by numerous authorities, both mathematicians and engineers, during the past seventeen years, and many series of experiments have been made under varying circumstances.

No formula has, however, yet been arrived at which can be universally accepted as superseding that on which the Tables are based, and the Author does not think any apology necessary for reproducing them as they are.

He has, however, endeavoured in the Introductory Chapter to make some comparison between them and the results obtained by other methods, and so to indicate more fully than he did before the limits within which they should be relied on for practical use.

6 DELAHAY STREET, WESTMINSTER.

February, 1901.

P R E F A C E.

IT has been found that the Engineering Pocket Books in most general use give comparatively little information relating to Sewerage and Water Supply. And even the large and valuable works of the late Mr. Beardmore and others contain somewhat abridged Tables applicable to the calculations most frequently required in designing and carrying out works of moderate size.

The Tables in this book have been calculated from time to time by the author to meet his own requirements. Thinking it probable that other engineers will have experienced the same want as himself, he has now been induced to make them public. The greater part have been used in manuscript for some years; but a few additional Tables have been recently added in order to make the work more complete.

Every precaution has been taken, as far as possible, to guard against errors both in the calculations and printing. If however, notwithstanding, any mistakes should be discovered, the author will be greatly obliged by having them pointed out to him.

6, DELAHAY STREET, WESTMINSTER,
November 1883.

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DESCRIPTION AND REMARKS ON
THE
USE OF THE TABLES.

TABLES I. and II. show the quantities of water in gallons per foot contained in pipes, wells, tanks, &c., of given dimensions, and require no explanation.

TABLES III. and IV. give the discharge in gallons per minute of water passing through sluices and over weirs under ordinary conditions. Correction is required in case of bell-mouthed or specially formed orifices, and also where there is any considerable velocity of current in approaching the outlets; but the notes at the heads of the Tables, to which attention should be directed, will enable this to be made with sufficient accuracy for most practical purposes.

TABLE V. shows the velocity and discharge under varying conditions of flow in circular sewers and conduits, from 9 inches to 6 feet in diameter.

In designing and carrying out sewerage works, it is important to know not only the maximum carrying

capacity of the sewers, but also the effect produced by the much smaller quantity which will be generally flowing through them. This is essential in order to ascertain whether flushing will be required, and if so, what quantity of water will be needed for the purpose. The Table consequently shows, not only the maximum discharge and velocity of each kind of sewer under the most favourable circumstances, but also the discharge and velocity of the same sewers when full to one-half, one-quarter, and one-eighth only of their heights respectively. If a sewer should at any time run quite full, its discharge will be somewhat less than that indicated in the fourth column, the velocity of current being in that case considerably diminished by friction against the top. With any circular conduit the velocity when full is exactly the same, and the discharge just double that when half-full ; the precise figures for a sewer running full may therefore be ascertained, if required, from the third column of Table by doubling the discharge.

A velocity of 150 feet per minute, or $2\frac{1}{2}$ feet per second, is generally considered sufficient to remove all obstacles of the ordinary character found in sewers. The quantity of water required to produce this velocity in each case is given in the last column

of the same Table, and will be found especially useful in designing flushing arrangements.

TABLE VI. gives precisely similar information for egg-shaped sewers, as Table V. for circular sewers.

TABLE VII. gives the discharge of pipes from $\frac{3}{8}$ -inch to 3 feet diameter, when running full at various inclinations or pressures. It should be remembered that the velocity of water passing through a line of pipes of any considerable length depends not on the inclination of any particular section, but on the hydraulic gradient throughout, or ratio of head of water to length of pipe; the "head" being the difference of level between the surface at or above the upper end of the pipe, and that of the cistern or pond into which it delivers, or if it has a free outlet, the lower end of the pipe itself. This velocity, except for slightly increased friction at bends, is entirely independent of the course of the pipes, whether laid at a uniform inclination or otherwise, also whether commencing at or below the upper surface and discharging, if not freely, at or below the lower surface.

The formula which has been used in the calculations for Tables V., VI. and VII. is that

known as Eytelwein's :— Velocity in feet per second = $94 \cdot 25 \sqrt{S}$, where R is the so-called "hydraulic mean depth," i.e. the sectional area divided by the surface in contact, and S the slope or inclination expressed fractionally, e.g. $\frac{1}{100}$ or $\frac{1}{250}$.

The constant number 94·25 has, of course, been arrived at as the result of experiments made from time to time in different kinds of pipes and channels with varying inclinations.

It has, however, long been known that this formula gives generally too high results for small pipes, and too low results for larger pipes and channels; and many other and more complicated formulæ have been from time to time devised in order to accord more nearly with more recent actual observations and experiments.

In addition to the alterations of flow due to the size, shape and inclination of channels, there is also considerable variation caused by the nature of the surface in contact with the water, in what degree it is smooth or rough.

The following Table gives some idea of the varying results that would be arrived at by using the coefficients or formulæ of different observers; the figures given being those which they would in each case substitute for the constant 94·25 used in the

Tables. When two figures are given, the difference is due to difference of inclination within moderate limits.

Diam. of Pipe running full or half full.	Darcy.			Kutter.	Professor Unwin.			Tables.
	For Clean Iron Pipes.	For Rusted Iron Pipes.	Mean	For Iron Pipes in Fair Condition.	For Clean Iron Pipes.	For In- crusted Pipes.	Mean	
2 in.	93	66	79	49·5 to 49				
3 „	98	69	83	57 „ 55				
6 „	105	74	89	71 „ 69	108 to 104	72	89	
12 „	109	77	93	87 „ 85	112 „ 109	76	93	
18 „	110	78	94	96 „ 94	116 „ 113	78	96	94·25
2 ft.	111	79	95	103 „ 101	120 „ 116	81	99	
3 „	111·5	79	95	111 „ 109	124 „ 120	83	102	
4 „	112	80	96	118 „ 116	128 „ 124	85	105	

It will be seen that, according to all the observations, the Tables will give correct results for pipes of a medium size, and too low results for larger ones ; excepting only in the case of incrusted iron pipes, for which the Tables are too high, even with the largest size.

Kutter's figures are calculated from a very elaborate formula,* containing a coefficient which may be

$$* \text{ Velocity in feet per second} = \frac{\sqrt{R}}{n} \frac{M + 1\cdot 811}{M + \sqrt{R}}, \text{ where}$$

$$M = n \left(41\cdot 6 + \frac{.00281}{S} \right), \text{ and } n \text{ for ordinary pipes} = .013.$$

In order to ascertain with facility the discharge of pipes from 2 to 48 inches in diameter, at varying inclinations, in accordance with this formula, Messrs. E. B. & G. M. Taylor have drawn and published a set of diagrams to a large scale showing curves from which they can be read off by inspection.

varied for different kinds of material, but the figures in the column above are those considered applicable to ordinary cast or wrought iron pipes, or to sewers or culverts of good brickwork or unglazed stoneware. For coated or enamelled iron pipes, or for glazed stoneware pipes, Kutter would use a multiplier giving somewhat higher figures.

As, however, sewers constructed of glazed pipes have necessarily joints not more than 3 feet apart and somewhat irregular, the Author is of opinion that they should be classed with ordinary rather than with specially smooth or enamelled pipes, and that, so far as Kutter's formula is correct, the figures in the Table should apply generally to sewers also.

The Author has himself experimented on the velocities in long lengths of a glazed pipe sewer 2 feet in diameter, running a third to a quarter full, at various inclinations, and has found that the formula on which the Tables are based, gives fairly accurate results in all cases. But when he had made similar trials in a 5-feet sewer, he found the Tables considerably too low. He has not had the opportunity of testing pipes running full, but as the water flowing in a 2-feet sewer one-third deep has the same hydraulic mean depth as that of a 15-inch sewer running full, he would conclude that in that

case also the Tables would be correct, although for sizes larger than 15 inches somewhat too low. This agrees approximately with Kutter.

With reference to pipes under 2 inches in diameter, both Darcy's and Kutter's coefficients would make the figures given in Table VII. much too high, but a series of experiments on lead pipes by Professor Osborne Reynolds showed them in fact only a little high, whereas another formula, Neville's,* makes them in some cases too low.

For pipes of this kind, whether iron or lead, in straight lines of considerable length, and known to be in perfect condition, the Author—on consideration of all the evidence so far recorded—would be disposed to make a small deduction from the Tables, say about 5 per cent. for one inch, and 10 per cent.

* Neville's formula, which has been largely used, and on which are based the Tables of Flow contained in Hurst's and Molesworth's Pocket-Books, is difficult to compare with others, as it shows the velocity composed of two parts, one proportional to the square roots, and the other to the cube roots, of the hydraulic mean depth and inclination. Thus, velocity in feet per second = $140 \sqrt{RS} - 11 \sqrt[3]{RS}$. This formula makes the flow in small pipes with considerable fall larger instead of smaller than the Tables—in fact, makes the Tables too low for $\frac{1}{2}$ -inch pipes steeper than 1 in 50, for 1-inch pipes steeper than 1 in 100, 3-inch steeper than 1 in 250, 6-inch steeper than 1 in 500, 12-inch steeper than 1 in 1250, 24-inch steeper than 1 in 3000, and for larger sizes, whatever the inclination, the greatest difference for 36-inch pipes being about 17 per cent. But for flatter gradients the Tables for all the smaller sizes are, according to this formula, too high.

for $\frac{1}{2}$ -inch diameters. But pipes of these dimensions as generally used for house services and similar purposes, are subject to so many irregularities, such as sharp bends, angles, contractions or other obstacles to flow, that a much greater deduction is, in practice, really always necessary. In fact, a better approximation to the actual discharge could generally be arrived at by calculating from a smaller diameter of pipe—say, by taking the mean between the figure in the Table for the required diameter, and that for the next size lower.

For iron pipes exceeding 3 inches diameter, if of the best kind, coated inside, or quite new and perfect, the Author would suggest an addition to the figures contained in Tables, varying generally from 5 per cent. for 6-inch to 15 per cent. for 36-inch diameters.

But for iron pipes not so good in condition, and generally for stoneware pipes or sewers running full or half-full, he would consider the Tables correct for diameters of either 12, 15 or 18 inches, according to circumstances; for smaller sizes than these he would make a small deduction, and for larger sizes an addition of about 5 per cent. for each foot in diameter.

As to flow in pipes and sewers running less than half-full, no general rule can be given applicable to varying depths and forms of section, without first

calculating the hydraulic mean depth; but it may be remarked that the hydraulic mean depth of a circular sewer running a quarter full will be approximately the same as that of one a little more than half the size half full, and that of one running an eighth full approximately the same as one of a little more than a quarter the size half full. But where sewage, not clear water, is the material to be dealt with, it is obvious that the flow in small pipes, or shallow channels, cannot be calculated with accuracy, as deposit on the sides and bottom may reduce the sectional area at any point very considerably.

TABLE VIII. is intended to assist in designing the capacity of sewers, and shows at a glance the quantity of sewage, irrespective of rain and surface water, which should be allowed for given populations. In certain cases (see note at foot of Table), the allowance for rain may also be calculated on the basis of population with the help of the last column of the Table, but under ordinary circumstances this should be taken in proportion to area, as shown by Table IX. next following.

TABLE IX. shows the quantity of water due to rainfall over given areas, and the quantities in gallons

per minute, when running off at different rates of flow. The latter columns of the Table are intended for calculating the capacity of sewers ; and the second and third columns for estimating the quantity of water that can be collected from areas and gathering grounds for irrigation or water supply. The areas dealt with range from 100 square feet (representing the roof of a small building) to one square mile.

TABLES X., XI., XII., are rainfall Tables, for the information contained in which the Author is indebted to Mr. H. Sowerby Wallis, who succeeded the late Professor Symons as the recorder of British Rainfall.

TABLES XIII. and XIV. are intended to facilitate the preparation of preliminary reports and rough estimates for works of water supply, and show the approximate dimensions of reservoirs, filter beds, main pipes, pumping machinery, &c., required for the supply of given populations. It is not of course asserted that the constant numbers assumed in the headings of the columns are universally applicable ; and some few, e.g. 100 feet lift to be pumped, are necessarily arbitrary. But the differences due to

variations in these conditions can be ascertained generally either by inspection or by a short calculation, and results may be thus arrived at with much greater facility than if the Tables were not available.

TABLE XV. gives results of analyses of potable waters. To engineers and others, not constantly or very frequently engaged in investigating the quality of water, the figures presented by an analysis convey little information without some readily available standard of comparison. This it is endeavoured to afford by means of this Table, which contains the results of analyses of well-known waters from nearly every description of source.

For many of these the Author is indebted to Dr. Voelcker; others are from analyses by Messrs. Dibdin, Campbell, Thresh, and other well-known chemists.

TABLES XVI. and XVII. give the quantities of brickwork per yard in sewers, culverts, &c., and require no explanation.

TABLE XVIII. gives the weight per yard of cast-iron pipes adapted to different pressures of water. These weights have been arrived at not by theoretical

calculation, but by a careful comparison of the specifications and recent practice of experienced engineers. They agree, however, nearly with the calculated strengths as given by Mr. Box in his Hydraulic Tables. The weights for various safe heads found in Table 14 of Beardmore's 'Manual of Hydrology,' are certainly insufficient according to recent practice.

TABLE XIX. gives the weights per yard of lead service pipes of five different qualities as described in the note appended to the Table.

TABLE I.—QUANTITY of WATER contained in PIPES, WELLS, and CIRCULAR TANKS, per foot in length or depth.

Diam. inches.	Contents gals. per foot	Diam. ft. in.	Contents. gals. per foot	Diam. feet.	Contents. gals. per foot	Diam. feet.	Contents. gala. per foot
$\frac{1}{2}$.005	1 9	15 0	11	594	90	39,758
$\frac{3}{4}$.008	2 0	19.6	12	77	100	49,088
$\frac{5}{8}$.019	2 3	24.8	13	829	110	59,396
1	.034	2 6	30.7	14	962	120	70,685
1 $\frac{1}{2}$.076	2 9	37.1	15	1,104	130	82,953
2	.135	3 0	44.2	16	1,256	140	96,211
2 $\frac{1}{2}$.212	3 3	51.8	17	1,418	150	110,447
3	.305	3 6	60.2	18	1,590	160	125,664
4	.54	3 9	69.0	19	1,772	170	141,862
5	.85	4 0	78.5	20	1,963	180	159,044
6	1.22	4 6	99.4	25	3,038	190	177,206
7	1.66	5 0	122.7	30	4,418	200	196,350
8	2.17	5 6	148.5	35	6,013	250	306,796
9	2.75	6 0	176.7	40	7,854	300	441,788
10	3.39	6 6	207.4	45	9,940	350	601,322
11	4.12	7 0	240.5	50	12,272	400	785,400
12	4.91	7 6	276.1	55	14,850	500	1,227,190
13	5.75	8 0	314.2	60	17,671	600	1,767,150
14	6.67	8 6	354.7	65	20,740	700	2,405,290
15	7.67	9 0	397.6	70	24,053	800	3,141,600
16	8.72	9 6	443.0	75	27,611	900	3,975,750
18	11.04	10 0	490.9	80	31,416	1000	4,908,750

TABLE II.—QUANTITY of WATER contained in SQUARE CISTERNS or TANKS, per foot in depth.

Length of Side.	Contents.	Length of Side.	Contents.	Length of Side.	Contents.	Length of Side.	Contents.
ft. in.	gals. per foot	ft. in.	gala. per foot	feet	gals per foot	feet	gals. per foot
1 0	6.25	6 0	205	25	3,906	90	50,625
1 6	14.06	7 0	306	30	5,625	100	62,500
2 0	25.00	8 0	400	35	7,756	125	156,250
2 6	39.06	9 0	506	40	10,000	150	140,625
3 0	56.25	10 0	625	45	12,656	200	250,000
3 6	77.56	11 0	756	50	15,625	300	562,500
4 0	100.00	12 0	900	60	20,500	400	1,000,000
4 6	126.56	15 0	1,406	70	30,625	500	1,562,500
5 0	156.25	20 0	2,500	80	40,000	1000	6,250,000

TABLE III.—Flow of WATER through SLUICES and OPENINGS.

NOTE.—The "Head of Water" in the Table must represent the depth from the surface to the centre of the opening; or if the opening be submerged, then the difference of level between the surfaces above and below.

If the opening be bell-mouthed, or be a sluice having curved side walls properly tapering inwards to the narrowest part, the discharge will be greater than that shown by the Table, to the extent of, in case of the best form of opening, about 50 per cent.

Head of Water. ft. in.	Discharge per Square Foot in Area of Opening. galls. per minute	Head of Water. ft. in.	Discharge per Square Foot in Area of Opening. galls. per minute	Head of Water. ft. in.	Discharge per Square Foot in Area of Opening. galls. per minute	Head of Water. ft. in.	Discharge per Square Foot in Area of Opening. gals. per minute
1 2	382	2 3	2,813	8 3	5,385	16 6	7,616
1	541	2 6	2,964	8 6	5,466	17 0	7,731
1 1/2	663	2 9	3,110	8 9	5,546	17 6	7,844
2	765	3 0	3,248	9 0	5,625	18 0	7,956
2 1/2	856	3 3	3,379	9 3	5,702	18 6	8,064
3	937	3 6	3,507	9 6	5,779	19 0	8,173
3 1/2	1,014	3 9	3,631	9 9	5,854	19 6	8,280
4	1,082	4 0	3,751	10 0	5,929	20 0	8,385
5	1,210	4 3	3,865	10 3	6,004	21 0	8,590
6	1,326	4 6	3,977	10 6	6,075	22 0	8,796
7	1,432	4 9	4,086	10 9	6,148	23 0	8,991
8	1,530	5 0	4,192	11 0	6,219	24 0	9,184
9	1,624	5 3	4,295	11 3	6,288	25 0	9,375
10	1,712	5 6	4,398	11 6	6,358	26 0	9,558
11	1,794	5 9	4,495	11 9	6,427	27 0	9,744
1 0	1,875	6 0	4,592	12 0	6,495	28 0	9,920
1 1	1,951	6 3	4,687	12 6	6,628	30 0	10,269
1 2	2,025	6 6	4,779	13 0	6,759	32 0	10,605
1 3	2,096	6 9	4,872	13 6	6,888	34 0	10,933
1 4	2,165	7 0	4,960	14 0	7,015	36 0	11,253
1 5	2,231	7 3	5,048	14 6	7,139	38 0	11,557
1 6	2,296	7 6	5,135	15 0	7,262	40 0	11,857
1 9	2,480	7 9	5,219	15 6	7,382	45 0	12,577
2 0	2,651	8 0	5,302	16 0	7,502	50 0	13,256

TABLE IV.—FLOW of WATER over WEIRS.

NOTE.—The "Depth" must represent difference in level between the sill of the weir and the surface of still water above it. If the water approaches the weir with a current having a perceptible velocity, the discharge will be greater than that shown by the Table to an extent depending on the velocity; a velocity of 2 feet per second will be equivalent generally to about half an inch, and a velocity of 3 feet per second to about three-quarters of an inch additional depth.

Dpth. inches	Discharge per Inch in Width.	Depth. inches	Discharge per Inch in Width.	Depth. inches	Discharge per Inch in Width.	Depth. ft. in.	Discharge per Inch in Width.
1 $\frac{1}{4}$	334	4 $\frac{1}{2}$	22.37	10 $\frac{1}{4}$	87.5	2 1	334
1 $\frac{3}{8}$	467	4 $\frac{1}{4}$	23.39	10 $\frac{1}{2}$	90.8	2 2	354
1 $\frac{5}{8}$	613	4 $\frac{3}{4}$	24.44	10 $\frac{3}{4}$	94.1	2 3	374
1 $\frac{1}{2}$	944	4 $\frac{1}{2}$	25.49	11	97.4	2 4	395
1 $\frac{5}{8}$	1329	4 $\frac{5}{8}$	26.56	11 $\frac{1}{4}$	100.7	2 5	417
1 $\frac{1}{4}$	1734	4 $\frac{3}{4}$	27.64	11 $\frac{1}{2}$	104.1	2 6	439
1 $\frac{7}{8}$	2185	4 $\frac{7}{8}$	28.74	11 $\frac{3}{4}$	107.5	2 7	461
1	2670	5	29.85	12	111.0	2 8	483
1 $\frac{1}{4}$	3185	5 $\frac{1}{8}$	30.97	12 $\frac{1}{2}$	118.0	2 9	506
1 $\frac{1}{2}$	3818	5 $\frac{1}{4}$	32.12	13	125.1	2 10	529
1 $\frac{5}{8}$	4305	5 $\frac{3}{4}$	33.26	13 $\frac{1}{2}$	132.5	2 11	553
1 $\frac{1}{2}$	4905	5 $\frac{1}{2}$	34.44	14	139.8	3 0	577
1 $\frac{5}{8}$	5531	5 $\frac{5}{8}$	35.62	14 $\frac{1}{2}$	147.4	3 1	601
1 $\frac{3}{4}$	6167	5 $\frac{3}{4}$	36.85	15	155.1	3 2	625
1 $\frac{7}{8}$	6855	5 $\frac{7}{8}$	38.02	15 $\frac{1}{2}$	163.0	3 3	650
2	7552	6	39.24	16	170.9	3 4	675
2 $\frac{1}{4}$	827	6 $\frac{1}{4}$	41.72	16 $\frac{1}{2}$	179.0	3 5	701
2 $\frac{1}{2}$	901	6 $\frac{1}{2}$	44.25	17	187.1	3 6	727
2 $\frac{3}{4}$	977	6 $\frac{3}{4}$	46.82	17 $\frac{1}{2}$	195.5	3 7	753
2 $\frac{1}{2}$	1055	7	49.45	18	203.9	3 8	779
2 $\frac{5}{8}$	1136	7 $\frac{1}{4}$	52.12	18 $\frac{1}{2}$	212.3	3 9	806
2 $\frac{3}{4}$	1218	7 $\frac{1}{2}$	54.84	19	221.1	3 10	833
2 $\frac{7}{8}$	1302	7 $\frac{3}{4}$	57.61	19 $\frac{1}{2}$	229.8	3 11	860
3	1387	8	60.41	20	238.8	4 0	888
3 $\frac{1}{4}$	1475	8 $\frac{1}{4}$	62.54	20 $\frac{1}{2}$	247.6	4 1	915
3 $\frac{1}{2}$	1564	8 $\frac{1}{2}$	66.17	21	256.9	4 2	944
3 $\frac{3}{4}$	1655	8 $\frac{3}{4}$	69.11	21 $\frac{1}{2}$	265.9	4 3	972
3 $\frac{1}{2}$	1748	9	72.09	22	275.5	4 4	1000
3 $\frac{5}{8}$	1842	9 $\frac{1}{4}$	75.12	22 $\frac{1}{2}$	284.8	4 6	1060
3 $\frac{3}{4}$	1939	9 $\frac{1}{2}$	78.18	23	294.4	4 8	1120
3 $\frac{7}{8}$	2037	9 $\frac{3}{4}$	81.29	23 $\frac{1}{2}$	303.9	4 10	1180
4	2136	10	84.43	24	313.9	5 0	1240

TABLE V.—VELOCITY and DISCHARGE per MINUTE in CIRCULAR SEWERS, with Water flowing at various depths.

VELOCITY and DISCHARGE per MINUTE in CIRCULAR SEWERS, with Water flowing at various depths.

Diameter 12 Inches.

Inclination.	Depth of Flow in Proportion to Height of Sewer.									
	One-eighth. (1½ Inch.)					One-quarter. (3 Inches.)				
	Velocity.	Discharge.	Velocity.	Discharge.	Velocity.	Discharge.	Velocity.	Discharge.	Velocity.	Discharge.
1 in	30	feet per mile	gallons	feet	gallons	feet	gallons	feet	gallons	feet
1 "	40	176	284	98	396	380	520	1,275	565	2580
1 "	50	132	247	86	342	330	446	1,100	490	2235
1 "	60	105·6	220	76	303	292	400	980	438	2000
1 "	66	80	192	66	268	260	348	850	380	1730
1 "	80	66	173	60	243	235	316	725	346	1580
1 "	100	52·8	155	53	220	212	282	690	309	1410
1 "	132	40	135	46	188	181	246	600	270	1230
1 "	165	32	121	42	169	162	220	540	241	1100
1 "	200	26·4	110	38	151	145	200	490	219	1000
1 "	264	20	96	33	134	130	174	425	190	865
1 "	330	16	85	29	119	115	155	380	170	780
1 "	440	12	74	25	103	99	135	331	147	670
1 "	528	10	67	23	94	90	123	300	135	615
1 "	660	8	60	21	84	81	110	270	120	550

VELOCITY and DISCHARGE per MINUTE in CIRCULAR SEWERS, with Water flowing at various depths.

Diameter 15 Inches.

Inclination.	Depth of Flow in Proportion to Height of Sewer.						Quantity required to give Velocity of 150 Feet per Minute.	
	One-eighth. ($\frac{1}{8}$ Inch.)		One-quarter. ($\frac{3}{4}$ Inches.)		One-half. ($\frac{7}{8}$ Inches.)			
	Velocity.	Discharge.	Velocity.	Discharge.	Velocity.	Discharge.		
feet per mile	gallons	feet	gallons	feet	gallons	feet	gallons	
1 in 40	132	278	150	385	592	500	547	
1 " 50	105.6	250	135	342	526	446	3900	
1 " 68	80	218	117	299	460	386	3480	
1 " 80	66	196	105	272	418	352	3030	
1 " 100	52.8	176	94	242	37.2	316	2760	
1 " 132	40	153	82	211	32.5	274	2460	
1 " 165	32	137	73	189	29.1	245	2140	
1 " 200	26.4	125	67	171	26.3	223	1910	
1 " 264	20	109	58	149	22.9	193	1516	
1 " 330	16	97	52	134	20.6	174	1350	
1 " 440	12	83	44	115	17.7	150	1175	
1 " 528	10	76	41	105	16.2	137	1068	
1 " 660	8	68	36	95	14.6	123	954	
1 " 880	6	60	32	82	12.6	105	824	

VELOCITY and DISCHARGE per MINUTE in CIRCULAR SEWERS, with Water flowing at various depths.

Diamster 18 Inches.

Inclination	Depth of Flow in Proportion to Height of Sewer.						Quantity required to give Velocity of 150 Feet per Minute.	
	One-eighth. (2½ Inches.)			One-quarter. (4½ Inches.)				
	Velocity	Discharge.	feet	Velocity	Discharge.	feet		
1 in 50	feet per mile	gallons	feet	gallons	feet	gallons	gallons	
1 " 66	105.6	210	382	830	488	2684	5500	
1 " 80	80	234	182	326	684	426	4776	
1 " 86	66	213	164	290	625	386	..	
1 " 100	52.8	190	147	265	573	346	4336	
1 " 132	40	166	129	230	497	301	3885	
1 " 165	32	148	115	208	450	268	54	
1 " 200	26.4	135	105	191	414	244	..	
1 " 264	20	117	91	163	310	213	38	
1 " 330	16	105	81	145	312	190	..	
1 " 440	12	91	70	126	272	165	..	
1 " 528	10	82	63	116	260	150	..	
1 " 660	8	73	57	104	225	135	1691	
1 " 880	6	65	50	89	192	116	807	
1 " 1056	5	58	45	81	170	106	1507	
2 " 1	528	10	82	63	260	150	147	
2 " 1	660	8	73	57	225	135	127	
2 " 1	880	6	65	50	192	116	106	
2 " 1	1056	5	58	45	170	106	..	
2 " 2	

VELOCITY and DISCHARGE per MINUTE in CIRCULAR SEWERS, with Water flowing at various depths.

Diameter 1 Foot 9 Inches.

VELOCITY and DISCHARGE per MINUTE in CIRCULAR SEWERS, with Water flowing at various depths.

Diameter 2 Feet.

Inclination.	Depth of Flow in Proportion to Height of Sewer.						Quantity required to give Velocity of 150 Feet per Minute.	
	One-eighth. (3 Inches.)		One-quarter. (6 Inches.)		One-half. (1 Foot.)			
	Velocity.	Discharge.	Velocity.	Discharge.	Velocity.	Discharge.		
	feet	gallons	feet	gallons	feet	gallons	gallons	
1 in	66	80	270	370	492	4820	9800	
1 " "	80	66	246	338	446	4370	8820	
1 " "	100	52.8	220	301	307	1182	3900	
1 " "	132	40	191	262	284	1092	438	
1 " "	165	32	171	234	239	920	3410	
1 " "	200	26.4	155	212	217	835	2764	
1 " "	264	20	135	185	189	728	246	
1 " "	330	16	121	166	169	650	220	
1 " "	440	12	105	145	146	562	190	
1 " "	528	10	96	131	134	515	174	
1 " "	660	8	85	116	119	458	155	
1 " "	880	6	74	101	103	396	134	
1 " "	1058	5	68	93	95	366	123	
1 " "	1320	4	60	82	84	323	110	

VELOCITY and DISCHARGE per MINUTE in CIRCULAR SEWERS, with Water flowing at various depths.

Diameter 2 Feet 3 Inches.

VELOCITY and DISCHARGE per MINUTE in CIRCULAR SEWERS, with Water flowing at various depths.

Diameter 2 Feet 6 Inches.

Inclination.	Depth of Flow in Proportion to Height of Sewer.					
	One-eighth. (3 <i>1</i> / ₄ Inches.)		One-quarter. (7 <i>1</i> / ₂ Inches.)		One-half. (1 Foot 3 Inches.)	
	Velocity.	Discharge.	Velocity.	Discharge.	Velocity.	Discharge.
feet per mile	feet	gallons	feet	gallons	feet	gallons
1 in	66	80	302	650	422	2520
1 "	100	52·8	246	529	344	2067
1 "	132	40	214	460	299	1797
1 "	165	32	191	411	267	1505
1 "	200	26·4	174	374	243	1460
1 "	264	20	151	825	211	1268
1 "	330	16	135	290	189	1136
1 "	440	12	117	251	164	986
1 "	528	10	107	230	150	901
1 "	660	8	96	206	134	805
1 "	880	6	82	176	115	691
1 "	1056	5	75	161	105	631
1 "	1320	4	68	146	94	565
1 "	1760	3	58	125	82	493
Quantity required to give Velocity of 150 Feet per Minute.						gallons
						17,150
						13,851
						12,141
						10,858
						9,832
						197
						8,678
						7,666
						6,640
						6,070
						5,416
						1380
						166
						150
						137
						123
						106
						116
						4,702
						4,275
						3,819
						3,320
						..

VELOCITY and DISCHARGE per MINUTE in CIRCULAR SEWERS, with Water flowing at various depths.

Diameter 2 Feet 9 Inches.

Inclination. feet per mile	Velocity. gallons feet	Depth of Flow in Proportion to Height of Sewer.				Quantity required to give Velocity of 150 Feet per Minute. gallons	
		One-eighth. (4 $\frac{1}{8}$ Inches.)		One-quarter. (8 $\frac{1}{4}$ in. hes.)			
		Velocity.	Discharge.	Velocity.	Discharge.		
1 in 66	80	316	822	444	3232	10,675	
1 " 100	52.8	258	671	360	2621	8,690	
1 " 132	40	224	582	313	2279	7,542	
1 " 166	32	200	520	280	2038	6,763	
1 " 200	26.4	183	476	255	1856	5,337	
1 " 264	20	158	411	222	1616	316	
1 " 330	16	142	369	198	1441	282	
1 " 440	12	124	322	172	1252	4,781	
1 " 528	10	112	291	157	1143	223	
1 " 660	8	100	260	140	1019	3,761	
1 " 880	6	87	226	121	881	173	
1 " 1056	5	79	207	110	801	2,928	
1 " 1320	4	71	185	99	753	144	
1 " 1760	3	62	166	86	626	158	
						122	
						1420	
						316	
						9,729	
						8,418	
						7,693	
						6,900	
						5,970	
						5,450	
						4,864	
						4,210	
						..	

VELOCITY and DISCHARGE per MINUTE in CIRCULAR SEWERS, with Water flowing at various depths.

Diameter 3 Feet.

VELOCITY and **DISCHARGE per MINUTE** in **CIRCULAR SEWERS**, with Water flowing at various depths.

Diameter 3 Feet 6 Inches.

Inclination.	Depth of Flow in Proportion to Height of Sewer.									
	One-eighth, (34 Loaves.)		One-quarter, (10½ Inches.)		One-half. (1 Foot 9 Inches.)		Seven-eighths. (Maximum Discharge.)		Quantity Required to give Velocity of 150 Feet per Minute.	
	Velocity.	Discharge.	Velocity.	Discharge.	Velocity.	Discharge.	Velocity.	Discharge.	gallons	gallons
1 in	66	80	359	1508	501	5887	651	19,530	713	39,860
1 ,	132	40	253	1062	355	4171	460	13,800	504	28,200
1 ,	200	26·4	206	865	288	3384	374	11,220	404	22,600
1 ,	264	20	179	752	251	2949	325	9,750	356	19,930
1 ,	330	16	160	672	224	2632	291	8,730	319	17,850
1 ,	440	12	139	584	194	2279	252	7,560	276	15,430
1 ,	528	10	126	529	177	2080	230	6,900	252	14,100
1 ,	660	8	113	475	158	1856	206	6,180	225	12,590
1 ,	880	6	98	412	136	1598	178	5,340	195	10,900
1 ,	1056	5	90	378	125	1469	162	4,860	178	9,960
1 ,	1320	4	80	336	112	1316	145	4,350	159	8,900
1 ,	1760	3	69	290	97	1140	126	3,780	138	7,720
1 ,	2112	2·5	63	265	88	1040	115	3,450	126	7,050
1 ,	2640	2	56	235	79	930	103	3,090	118	6,320

VELOCITY and **DISCHARGE** per MINUTE in CIRCULAR SEWERS, with Water flowing at various depths.

Diameter 4 Feet.

Inclination.	Depth of Flow in Proportion to Height of Sewer.						gallons	
	One-eighth. (6 Inches.)			One-quarter. (1 Foot.)				
	Velocity.	Discharge.	Velocity.	Discharge.	Velocity.	Discharge.		
feet per mile	feet	gallons	feet	gallons	feet	gallons	feet	
1 in	66	80	384	2110	536	8240	695	
1 "	132	40	271	1490	372	5720	492	
1 "	200	26.4	220	1210	302	4640	400	
1 "	264	20	192	1055	268	4120	348	
1 "	330	16	171	940	238	3658	310	
1 "	440	12	148	814	204	3136	269	
1 "	528	10	134	737	186	2860	246	
1 "	660	8	121	665	166	2550	220	
1 "	880	6	105	577	146	2244	190	
1 "	1056	5	96	528	134	2059	174	
1 "	1320	4	86	473	119	1829	155	
1 "	1760	3	74	407	102	1568	134	
1 "	2112	2.5	67	368	93	1430	123	
1 "	2640	2	60	330	83	1275	110	

VELOCITY and DISCHARGE per MINUTE in CIRCULAR SEWERS, with Water flowing at various depths.

Diameter 5 Feet.

WATER FLOWING AT VARIOUS DEPTHS.

Diameter 6 Feet.

Inclination.	Depth of Flow in Proportion to Height of Sewer.						Quantity required to give Velocity of 150 Feet per Minute.				
	One-eighth. (9 Inches.)		One-quarter. (1 Foot 6 Inches.)		One-half. (3 Feet.)						
	Velocity.	Discharge.	Velocity.	Discharge.	Velocity.	Discharge.					
1 in	66	80	468	5790	652	22,580	852	75,200	932	153,000	..
1 , ,	132	40	332	4110	462	16,000	602	53,120	660	108,400	..
1 , ,	200	26·4	270	3340	382	13,140	488	43,060	536	88,040	..
1 , ,	264	20	234	2895	326	11,290	426	37,600	466	76,500	455
1 , ,	330	16	210	2610	290	10,040	380	33,535	418	68,660	640
1 , ,	440	12	182	2250	252	8,720	330	29,120	360	59,130	980
1 , ,	528	10	166	2055	232	8,000	301	26,560	330	54,200	1,320
1 , ,	660	8	148	1830	208	7,200	270	23,830	294	48,290	1,890
1 , ,	880	6	129	1600	178	6,160	232	20,480	254	41,740	2,950
1 , ,	1056	5	117	1448	162	5,645	212	18,800	233	38,250	3,850
1 , ,	1320	4	105	1300	145	5,020	190	16,770	209	34,330	5,670
1 , ,	1760	3	91	1126	126	4,360	165	14,560	180	29,560	9,340
1 , ,	2112	2·5	83	1027	116	4,000	150	13,280	165	27,100	13,200
1 , ,	2640	2	74	917	104	3,600	135	11,915	147	24,140	..

TABLE VI.—VELOCITY and DISCHARGE per MINUTE in EGG-SHAPED SEWERS, with Water flowing at various depths.

Sewer 2 Feet x 1' Foot 4 Inches.

VELOCITY and **DISCHARGE** per MINUTE in EGG-SHAPED SEWERS, with Water flowing at various depths.

Sewer 2 Feet 3 Inches x 1 Foot 6 Inches.

VELOCITY and DISCHARGE per MINUTE in Egg-shaped SEWERS, with Water flowing at various depths.

Sewer 2 Feet 6 Inches × 1 Foot 8 Inches.

Inclination.	Depth of Flow in proportion to Height of Sewer.						Quantity required to give Velocity of 150 Feet per Minute.	
	One-eighth. ($\frac{3}{4}$ inches.)			One-quarter. ($\frac{7}{8}$ inches.)				
	Velocity.	Discharge.	Velocity.	Discharge.	Velocity.	Discharge.		
1 in	66	80	280	338	371	1203	feet gallons feet gallons feet gallons feet gallons	
1 " 100	52	8	226	272	301	972	467 +138 522 9500	
1 " 132	40	198	238	261	846	369 3350	424 7700	
1 " 165	32	176	214	236	764	292+ 330	369 6700	
1 " 200	26	4	160	193	212	687	2620 300	6000 5450
1 " 264	20	140	169	186	601	233	2069 261	4750 210
1 " 330	16	124	150	165	534	209	1852 235	4280 335
1 " 440	12	108	131	143	463	180	1598 202	3670 600
1 " 528	10	99	120	131	424	165	1462 185	3350 890
1 " 660	8	88	107	118	382	148	1311 165	3000 1500
1 " 880	6	77	93	101	328	128	1132 143	2600 ..
1 " 1056	5	70	84	92	300	117	103+ 131	2380 ..
1 " 1320	4	62	74	82	266	105	926 118	2140 ..
1 " 1760	3	54	65	71	230	90	800 101	1834 ..

VELOCITY and DISCHARGE per MINUTE in EGG-SHAPED SEWERS, with Water flowing at various depths.

Sewer 2 Feet 9 Inches × 1 Foot 10 Inches.

Inclination. feet per mile	Depth of Flow in Proportion to Height of Sewer.						Quantity required to give Velocity of 150 Feet per Minute. gallons	
	One-eighth. (4½ Inches.)			One-quarter. (8½ Inches.)				
	Velocity.	Discharge.	Velocity.	Discharge.	Velocity.	Discharge.		
1 in 66 80	feet	gallons	feet	gallons	feet	gallons	12,050	
1 " 100 52.8	300	432	387	1,518	489	5230	550	
1 " 132 40	243	350	313	1,230	402	4300	446	
1 " 165 32	212	305	274	1,077	345	3690	389	
1 " 200 26.4	190	274	244	956	308	3300	348	
1 " 264 20	172	248	222	870	284	3040	316	
1 " 330 16	150	216	194	760	244	2610	274	
1 " 440 12	134	192	172	674	218	2333	246	
1 " 528 10	116	168	150	588	190	2033	214	
1 " 680 8	106	153	137	538	172	1840	194	
1 " 880 6	95	137	122	478	154	1650	174	
1 " 1056 5	82	118	106	411	133	1420	150	
1 " 1320 4	75	108	97	380	122	1310	137	
1 " 1760 3	67	96	86	337	109	1166	123	
	58	84	75	294	95	1016	107	
							1440	
							3,860	
							130	
							3300	
							3,300	
							3,010	
							2,700	
							2,350	
							..	

VELOCITY and **DISCHARGE per MINUTE** in Egg-shaped SEWERS, with Water flowing at various depths.

Sewer 3 Feet x 2 Feet.

Inclination.	Depth of Flow in Proportion to Height of Sewer.						Quantity required to give Velocity of 150 Feet per Minute.	
	One-eighth. (4½ Inches.)		One-quarter. (9 Inches.)		One-half. (1 Foot 6 Inches.)			
	Velocity.	Discharge.	Velocity.	Discharge.	Velocity.	Discharge.		
1 in	66	80	313	404	510	6500	574 14,900	
1 "	100	52·8	255	437	414	5280	467 12,120	
1 "	132	40	221	380	361	4600	407 10,550	
1 "	165	32	198	338	324	4130	364 9,450	
1 "	200	26·4	180	309	293	3735	330 8,570	
1 "	264	20	157	270	940	3250	286 7,450	
1 "	330	16	139	238	840	2910	228 7,450	
1 "	440	12	121	208	728	2525	222 6,680	
1 "	528	10	111	190	668	2300	203 5,770	
1 "	660	8	99	169	600	2065	182 5,270	
1 "	880	6	86	147	517	1785	157 4,075	
1 "	1056	5	78	135	470	1620	148 3,730	
1 "	1320	4	70	120	420	1455	128 3,340	
1 "	1760	3	61	105	78	1262	111 2,885	

VELOCITY and DISCHARGE per MINUTE in EGG-SHAPED SEWERS, with Water flowing at various depths,

Sewer 3 Feet 3 Inches \times 2 Feet 2 Inches.

Inclination.	Depth of Flow in Proportion to Height of Sewer.						Quantity required to give Velocity of 150 Feet per Minute.	
	One-eighth. (4 $\frac{1}{8}$ Inches.)		One-quarter. (9 $\frac{1}{4}$ Inches.)		One-half. (1 Foot 7 $\frac{1}{4}$ Inches.)			
	Velocity.	Discharge.	Velocity.	Discharge.	Velocity.	Discharge.		
	feet	gallons	feet	gallons	feet	gallons	gallons.	
1 in 66	80	326	655	421	2300	532	7975	
1 " 100	52.8	264	531	341	1865	432	6475	
1 " 132	40	230	462	298	1630	376	5635	
1 " 165	32	207	416	266	1455	336	5040	
1 " 200	26.4	186	374	241	1320	304	4560	
1 " 264	20	161	324	210	1150	266	3990	
1 " 330	16	143	287	187	1023	238	3565	
1 " 440	12	126	253	164	897	206	3090	
1 " 528	10	115	231	149	825	187	2800	
1 " 660	8	103	207	133	727	168	2520	
2 " 880	6	89	179	115	630	145	2170	
2 " 1056	5	81	163	105	574	133	1995	
2 " 1320	4	71	144	93	511	119	1785	
2 " 1760	3	63	127	82	448	103	1540	

2

Velocity and Discharge per Minute in Egg-shaped Sewers, with Water flowing at various depths.

Sewer 3 Feet 6 Inches \times 2 Feet 4 Inches.

Inclination.	Depth of Flow in Proportion to Height of Sewer.						Quantity required to give Velocity of 150 Feet per Minute.	
	One-eighth. ($\frac{5}{8}$ Inches.)		' One-half. (1 Foot 9 Inches.)		Seven-eighths. (Maximum Discharge.)			
	Velocity.	Discharge.	Velocity.	Discharge.	Velocity.	Discharge.		
feet per mile	feet	gallons	feet	gallons	feet	gallons	gallons	
1 in 100	52.8	275	642	355	2260	448	17,950	
1 " 132	40	240	560	300	1900	390	16,660	
1 " 165	32	214	500	276	1740	350	14,030	
1 " 200	26.4	195	455	251	1600	317	12,700	
1 " 264	20	170	396	218	1370	275	11,100	
1 " 330	16	152	355	196	1240	247	9,900	
1 " 440	12	132	308	170	1080	215	8,600	
1 " 528	10	120	280	154	950	195	7,830	
1 " 660	8	107	250	138	870	175	7,015	
1 " 880	6	93	217	120	760	151	6,050	
1 " 1056	5	85	198	109	690	138	5,500	
1 " 1320	4	76	177	98	623	124	4,950	
1 " 1760	3	66	154	85	540	108	4,800	
1 " 2640	2	53	124	69	437	87	3,510	

VELOCITY and **DISCHARGE per MINUTE** in EGG-SHAPED SEWERS, with Water flowing at various depths.

Sewer 3 Feet 9 Inches x 2 Feet 6 Inches.

Inclination.	Depth of Flow in Proportion to Height of Sewer.									
	One-eighth. (58 Inches.)					One-quarter. (11½ Inches.)				
	Velocity.	Discharge.	Velocity.	Discharge.	Velocity.	One-half. (1 Foot 10½ Inches.)	Discharge.	Velocity.	Seven-eighths. (Maximum Discharge.)	gallons
feet per mile	feet	gallons	feet	gallons	feet	feet	gallons	feet	feet	gallons
1 in 100	62·8	284	758	367	2665	464	9190	521	21,200	..
1 , , 132	40	248	662	319	2315	404	8000	454	18,460	80
1 , , 165	32	222	592	286	2075	360	7130	405	16,470	115
1 , , 200	26·4	201	536	260	1890	328	6495	369	15,000	145
1 , , 284	20	175	467	226	1640	285	5645	321	13,050	225
1 , , 330	16	157	418	201	1460	255	5050	287	11,670	360
1 , , 440	12	136	362	175	1270	221	4375	249	10,125	610
1 , , 528	10	124	331	160	1160	202	4000	227	9,230	865
1 , , 660	8	111	296	143	1038	180	3565	203	8,240	1350
1 , , 880	6	96	256	124	901	156	3090	176	7,155	2550
1 , , 1056	5	87	234	113	820	143	2830	160	6,520	3850
1 , , 1320	4	78	209	101	730	127	2525	143	5,825	..
1 , , 1760	3	68	181	87	635	110	2188	124	5,060	..
1 , , 2640	2	55	148	71	515	90	1782	102	4,120	..

Velocity and Discharge per Minute in Egg-shaped Sewers, with Water flowing at various depths.

Sewer 4 Feet \times 2 Feet 8 Inches.

Inclination,	Depth of Flow in Proportion to Height of Sewer.						Quantity required to give Velocity of 150 Feet per Minute.	
	One-eighth. (6 Inches.)			One-quarter. (1 Foot.)				
	Velocity.	Discharge.	Velocity.	Discharge.	Velocity.	Discharge.		
feet per mile	feet	gallons	feet	gallons	feet	gallons	gallons	
1 in 100	52.8	294	84	380	3150	479	25,000	
1 " 182	40	255	780	330	2740	417	21,760	
1 " 165	32	226	680	295	2450	372	19,500	
1 " 200	26.4	208	635	268	2220	339	17,870	
1 " 264	20	181	550	234	1940	295	15,430	
1 " 330	16	162	490	208	1725	264	13,800	
1 " 440	12	140	430	180	1500	228	11,900	
1 " 528	10	128	390	165	1350	208	10,880	
1 " 660	8	113	340	148	1230	186	9,750	
1 " 880	6	99	300	128	1065	162	8,460	
1 " 1056	5	90	275	117	970	148	7,720	
1 " 1320	4	81	245	104	863	132	6,900	
1 " 1760	3	70	210	90	750	114	5,950	
1 " 2640	2	57	170	74	615	93	4,880	

VELOCITY and DISCHARGE per MINUTE in EGG-SHAPED SEWERS, with Water flowing at various depths.

Sewer 4 Feet 6 Inches x 3 Feet,

Inclination.	Depth of Flow in Proportion to Height of Sewer.					
	One-eighth. (6 <i>½</i> Inches.)		One-quarter. (1 Foot 1 <i>½</i> Inch.)		One-half. (2 Feet 3 Inches.)	
	Velocity.	Discharge.	Velocity.	Discharge.	Velocity.	Discharge.
1 in 100	52.8	314	1230	402	508	14,540
1 " 132	40	271	1050	350	442	12,650
1 " 165	32	240	925	314	3360	11,320
1 " 200	26.4	223	860	284	3040	10,300
1 " 264	20	192	740	248	2655	8,930
1 " 330	16	172	664	222	2375	280
1 " 440	12	148	572	192	2055	242
1 " 528	10	136	525	175	1870	221
1 " 660	8	120	463	157	1680	198
1 " 880	6	105	405	136	1455	171
1 " 1056	5	96	372	124	1330	156
1 " 1320	4	86	334	111	1190	140
1 " 1760	3	74	286	96	1030	121
1 " 2640	2	60	232	78	840	99
						2,834
						111
Quantity required to give Velocity of 150 Feet per Minute.						gallons
						33,500
						29,250
						26,130
						23,830
						20,720
					feet	feet
					314	18,480
					272	370
					248	620
					222	860
					192	1350
					171	2400
					156	10,360
					140	9,240
					121	8,000
					99	6,530
				

VELOCITY and **DISCHARGE per MINUTE** in EGG-SHAPED SEWERS, with Water flowing at various depths.

Sewers 5 Feet x 3 Feet 4 Inches.

VELOCITY and DISCHARGE per MINUTE in EGG-SHAPED SEWERS, with Water flowing at various depths.

Sewers 6 Feet \times 4 Feet.

Inclination.	Depth of Flow in Proportion to Height of Sewer.						Quantity required to give Velocity of 150 Feet per Minute.	
	One-eighth. (9 Inches.)			One-quarter. (1 Foot 6 Inches.)				
	Velocity.	Discharge.	Velocity.	Discharge.	Velocity.	Discharge.		
feet per mile			feet	gallons	feet	gallons	gallons	
1 in 100	52.8	357	2451	462	8628	29,700	68,410	
1 " 132	40	313	2148	401	7488	25,984	59,938	
1 " 165	32	278	1910	360	6720	23,230	53,560	
1 " 200	26.4	254	1744	327	6106	21,093	48,746	
1 " 264	20	221	1517	286	5341	18,342	42,365	
1 " 330	16	198	1359	255	4762	16,406	37,970	
1 " 440	12	171	1174	221	4127	14,215	32,800	
1 " 528	10	156	1072	201	3753	12,992	29,917	
1 " 660	8	139	954	180	3361	11,616	26,780	
1 " 880	6	121	830	156	2913	10,037	25,314	
1 " 1066	5	110	755	143	2670	180	21,130	
1 " 1320	4	99	679	127	2372	161	18,933	
1 " 1760	3	85	583	110	2054	140	16,318	
1 " 2640	2	69	474	90	1681	114	13,389	

TABLE VII.—DISCHARGE of PIPES (running full).

Note.—The velocity in feet per minute may be ascertained in each case by dividing the discharge by the number of gallons contained in each lineal foot of the pipe as given at the top of the column.

Ratio of Head of Water to Length of Pipe.	Diameter of Pipe.					
	4 Inch. (.005 Galls. per Ft.)	4 Inch. (.008 Galls. per Ft.)	4 Inch. (.019 Galls. per Ft.)	1 Inch. (.034 Galls. per Ft.)	1½ Inch. (.053 Galls. per Ft.)	2 Inches. (.135 Galls. per Ft.)
1 to 1	2.39	4.91	13.52	27.75	48.55	76.66
1 " 2	1.70	3.47	9.56	19.63	34.32	54.23
1 " 3	1.38	2.85	7.86	16.13	28.20	44.54
1 " 4	1.19	2.46	6.76	13.87	24.27	38.33
1 " 5	1.07	2.20	6.05	12.40	21.70	34.28
1 " 6	.97	2.00	5.52	11.33	19.81	31.29
1 " 7	.90	1.85	5.10	10.47	18.32	28.93
1 " 8	.85	1.73	4.78	9.81	17.15	27.09
1 " 9	.80	1.64	4.51	9.25	16.18	25.55
1 " 10	.75	1.55	4.28	8.78	15.36	24.26
1 " 12	.69	1.42	3.91	8.02	14.30	22.16
1 " 14	.64	1.32	3.62	7.44	13.00	20.50
1 " 16	.60	1.23	3.38	6.94	12.14	19.16
1 " 18	.56	1.17	3.19	6.53	11.44	18.10
1 " 20	.53	1.10	3.03	6.21	10.85	17.15
						35.2
						61.3

DISCHARGE OF PIPES (running full).

Note.—The velocity in feet per minute may be ascertained in each case by dividing the discharge by the number of gallons contained in each lineal foot of the pipe as given at the top of the column.

Ratio of Head of Water to Length of Pipe.	Diameter of Pipe.						
	$\frac{1}{4}$ Inch. (.005 Galls. per Ft.)	$\frac{1}{2}$ Inch. (.008 Galls. per Ft.)	$\frac{3}{4}$ Inch. (.019 Galls. per Ft.)	1 Inch. (.034 Galls. per Ft.)	$1\frac{1}{4}$ Inch. (.053 Galls. per Ft.)	2 Inches. (.135 Galls. per Ft.)	
1 to 25	.48	.98	2.71	5.55	9.70	15.33	31.4
1 " 30	.44	.90	2.48	5.08	8.90	14.05	29.3
1 " 35	.40	.83	2.28	4.69	8.20	12.95	26.5
1 " 40	.38	.78	2.14	4.40	7.70	12.12	24.9
1 " 45	.36	.73	2.02	4.14	7.23	11.42	23.4
1 " 50	.33	.69	1.92	3.93	6.86	10.80	22.2
1 " 60	.31	.64	1.76	3.60	6.30	9.90	20.4
1 " 70	.28	.59	1.62	3.32	5.80	9.16	18.8
1 " 80	.27	.55	1.50	3.10	5.40	8.60	17.5
1 " 100	.24	.49	1.34	2.77	4.86	7.66	15.7
1 " 120	.21	.44	1.23	2.52	4.40	6.95	14.3
1 " 150	.19	.40	1.11	2.27	3.96	6.26	12.8
1 " 200	.17	.35	.96	1.96	3.43	5.42	11.1
1 " 250	.15	.31	.85	1.75	3.07	4.85	9.9
1 " 300	.14	.29	.79	1.61	2.82	4.45	9.1

 $\frac{1}{4}$ Inch.
(.005 Galls.
per Ft.) $\frac{1}{2}$ Inch.
(.008 Galls.
per Ft.) $\frac{3}{4}$ Inch.
(.019 Galls.
per Ft.)1 Inch.
(.034 Galls.
per Ft.) $1\frac{1}{4}$ Inch.
(.053 Galls.
per Ft.)2 Inches.
(.135 Galls.
per Ft.) $2\frac{1}{4}$ Inches.
(.212 Galls.
per Ft.)

DISCHARGE OF PIPES (running full).

NOTE.—The velocity in feet per minute may be ascertained in each case by dividing the discharge by the number of gallons contained in each lineal foot of the pipe as given at the top of the column.

Ratio of Head of Water to Length of Pipe.	Diameter of Pipe.					
	3 Inches. (*305 Galls. per Ft.)	4 Inches. (*54 Galls. per Ft.)	5 Inches. (*85 Galls. per Ft.)	6 Inches. (1·22 Galls. per Ft.)	7 Inches. (1·66 Galls. per Ft.)	8 Inches. (2·17 Galls. per Ft.)
1 to 5	193	398	695	1097	1613	2253
1 " 10	137	281	491	776	1140	1592
1 " 15	112	230	401	633	931	1300
1 " 20	97	199	347	548	806	1126
1 " 25	86	178	311	491	721	1007
1 " 30	79	162	283	448	658	920
1 " 35	73	150	263	415	610	851
1 " 40	68	141	246	388	570	796
1 " 45	64	133	232	366	538	751
1 " 50	61	126	222	347	510	712
1 " 60	56	115	201	317	466	650
1 " 70	52	106	186	293	431	594
1 " 80	49	99	174	274	403	563
1 " 90	46	94	164	258	380	536
1 " 100	43	89	155	245	360	503

10 Inches.
(3·39 Galls.
per Ft.)

gall., per min.
3933

2780

2270

1967

1759

3020

2138

1745

1511

1234

1142

1069

1007

1606

1487

1391

1311

873

808

756

712

1051

983

927

879

DISCHARGE OF PIPES (running full).

Note.—The velocity in feet per minute may be ascertained in each case by dividing the discharge by the number of gallons contained in each lineal foot of the pipe as given at the top of the column.

Ratio of Head of Water to Length of Pipe.	Diameter of Pipe.					
	3 Inches. (.305 Gall. per Ft.)	4 Inches. (.54 Gall. per Ft.)	5 Inches. (.85 Gall. per Ft.)	6 Inches. (1.22 Gall. per Ft.)	7 Inches. (1.66 Gall. per Ft.)	8 Inches. (2.17 Gall. per Ft.)
1 „ 125	39	80	139	219	323	450
1 „ 150	36	73	127	200	296	411
1 „ 175	33	67	117	183	273	380
1 „ 200	31	62	109	173	262	352
1 „ 250	27	56	98	154	227	317
1 „ 300	25	51	90	142	208	291
1 „ 350	23	47	83	131	193	270
1 „ 400	21	44	78	123	180	252
1 „ 450	20	42	73	116	170	238
1 „ 500	19	40	69	110	161	225
1 „ 600	18	36	63	100	147	206
1 „ 700	17	34	59	93	136	191
1 „ 800	16	31	55	87	127	178
1 „ 900	15	29	52	82	120	168
1 „ 1000	14	28	49	78	114	159

10 Inches.
(3.39 Gall.
per Ft.)

galls. per min.

786

552

718

665

622

554

426

360

508

470

440

415

393

DISCHARGE OF PIPES (running full).

NOTE.—The velocity in feet per minute may be ascertained in each case by dividing the discharge by the number of gallons contained in each lineal foot of the pipe as given at the top of the column.

Ratio of Head of Water to Length of Pipe.	'Diameter of Pipe.					
	12 Inches. (4.91 Galls. per Ft.)	15 Inches. (7.67 Galls. per Ft.)	18 Inches. (11.04 Galls. per Ft.)	21 Inches. (15 Galls. per Ft.)	24 Inches. (19.6 Galls. per Ft.)	27 Inches. (24.8 Galls. per Ft.)
1 to 20	3,103	5,420	8,551	12,570	17,552	23,360
1 " 25	2,775	4,848	7,648	11,240	15,698	21,070
1 " 30	2,533	4,426	6,982	10,262	14,330	19,235
1 " 40	2,194	3,833	6,047	8,888	12,411	16,660
1 " 50	1,962	3,428	5,408	7,950	11,100	14,900
1 " 60	1,792	3,130	4,937	7,257	10,133	13,600
1 " 70	1,660	2,897	4,571	6,717	9,382	12,593
1 " 80	1,551	2,710	4,276	6,284	8,776	11,943
1 " 90	1,462	2,555	4,032	5,925	8,274	11,105
1 " 100	1,387	2,424	3,824	5,621	7,850	10,535
1 " 125	1,241	2,168	3,420	5,027	7,021	9,423
1 " 150	1,133	1,980	3,123	4,591	6,411	8,605
1 " 175	1,049	1,832	2,890	4,250	5,933	7,964
1 " 200	981	1,714	2,698	3,974	5,538	7,450
1 " 250	874	1,527	2,410	3,542	4,946	6,638
						8,640
						13,628
						48,365
						43,265
						39,490
						34,200
						30,588

DISCHARGE OF PIPES (running full).

NOTE.—The velocity in feet per minute may be ascertained in each case by dividing the discharge by the number of gallons contained in each lineal foot of the pipe as given at the top of the column.

Ratio of Head of Water to Length of Pipe.	Diameter of Pipe.					
	12 Inches. (4.91 Galls. per Ft.)	15 Inches. (7.67 Galls. per Ft.)	18 Inches. (11.04 Galls. per Ft.)	21 Inches. (15 Galls. per Ft.)	24 Inches. (19.6 Galls. per Ft.)	27 Inches. (24.8 Galls. per Ft.)
1 to 300.	801	1,400	2,208	3,245	4,532	6,083
1 " 350	742	1,296	2,044	3,004	4,196	5,567
1 " 400	694	1,212	1,912	2,810	3,925	5,268
1 " 450	654	1,143	1,803	2,650	3,700	4,966
1 " 500	620	1,084	1,710	2,514	3,510	4,712
1 " 600	566	990	1,561	2,295	3,204	4,300
1 " 700	524	916	1,445	2,124	2,971	3,982
1 " 800	490	857	1,352	1,987	2,775	3,725
1 " 900	462	808	1,275	1,873	2,616	3,512
1 " 1,000	439	766	1,210	1,777	2,482	3,332
1 " 1,250	392	684	1,081	1,590	2,220	2,980
1 " 1,500	358	627	987	1,451	2,027	2,720
1 " 2,000	310	542	855	1,257	1,755	2,356
1 " 3,000	253	443	698	1,026	1,433	1,924
1 " 5,000	196	343	541	795	1,110	1,490
						gall. per min.
						30 Inches. (30.7 Galls. per Ft.)
						36 Inches. (44.2 Galls. per Ft.)

TABLE VIII.—QUANTITY of SEWAGE due to POPULATION.

Population.	Average Flow during 24 hours.			Maximum Flow, half in 6 hours.			Allowance for Rainfall for Population of 100 per acre, or 435 super. feet of area per inhabitant.		
	At 20 Galls. per Head.	At 30 Galls. per Head.	At 50 Galls. per Head.	At 20 Galls. per Head.	At 30 Galls. per Head.	At 50 Galls. per Head.	At $\frac{1}{2}$ Inch in 24 Hours.	At $\frac{1}{2}$ Inch in 24 Hours.	At 1 Inch in 24 hours.
500	7	10	17	14	21	35	19.6	39.3	78.7
1,000	14	21	35	28	42	69	39	79	157
2,000	28	42	69	56	83	139	79	157	315
3,000	42	62	104	83	125	208	118	236	472
4,000	56	83	139	111	167	278	157	315	629
5,000	69	104	174	139	208	347	196	393	787
6,000	83	125	208	167	250	417	235	472	944
7,000	97	146	243	194	292	486	275	551	1,101
8,000	111	167	278	222	338	556	314	630	1,258
9,000	125	187	312	250	375	625	353	708	1,416
10,000	139	208	347	278	417	694	393	787	1,573
20,000	278	417	694	555	833	1,389	787	1,573	3,146
30,000	416	625	1,041	833	1,250	2,083	1,179	2,358	4,717
40,000	555	833	1,389	1,110	1,667	2,778	1,573	3,146	6,292
50,000	694	1,042	1,736	1,589	2,083	3,472	1,966	3,932	7,865

QUANTITY of SEWAGE due to POPULATION.

Population.	Average Flow during 24 hours.			Maximum Flow, half in 6 hours.			Allowance for Rainfall for Population of 100 per acre, or 435 super. feet of area per inhabitant.		
	At 20 Galls. per Head.	At 30 Galls. per Head.	At 50 Galls. per Head.	At 20 Galls. per Head.	At 30 Galls. per Head.	At 50 Galls. per Head.	At $\frac{1}{2}$ Inch in 24 Hours.	At $\frac{1}{2}$ Inch in 24 Hours.	At 1 Inch in 24 Hours.
	galls. per min.	galls. per min.	galls. per min.	galls. per min.	galls. per min.	galls. per min.	galls. per min.	galls. per min.	galls. per min.
60,000	833	2,083	1,250	1,666	2,500	4,166	2,358	4,717	9,434
70,000	972	2,430	1,458	1,944	2,916	4,860	2,652	5,504	11,009
80,000	1,110	1,667	2,778	2,220	3,334	5,556	3,146	6,292	12,584
90,000	1,250	1,875	3,125	2,500	3,750	6,250	3,539	7,079	14,157
100,000	1,389	2,083	3,472	2,778	4,166	6,944	3,932	7,865	15,729

250 gallons per inhabited house, being about 44 gallons per head, is the quantity prescribed by Act of Parliament to be provided for in the Lower Thames Valley and Darent Valley Main Sewerage Districts. This is understood to include some allowance for rainfall.

Rainfall should not be taken on the basis of population, as in the third column, unless either the whole area to be provided for is continuously built upon, or the separate system is adopted and rain not admitted to the sewers except in close proximity to houses.

In the former case, if the population be greater than is assumed, the figures in the Table must obviously be divided by the ratio to 100; thus, for population of 200 per acre divide by 2, for 150 per acre take two-thirds, &c., and similarly for 50 per acre multiply by 2, &c.

On the other hand, if the system to be adopted is that of excluding the rain water, the average area pertaining to each inhabited house must first be ascertained and the number of persons per house; and the figures in the third column may be adopted or will require modification, according as the result arrived at compares with the assumption of 435 super feet to each individual.

TABLE IX.—QUANTITY and DISCHARGE from AREAS due to RAINFALL.

Area.	Quantity equal to 1 Inch of Rain over Surface.	Equivalent Supply Daily throughout the Year.	Quantity running off at following Rates.					
			1 Inch in an hour.	1 Inch in an hour.	1 Inch in an hour.	1 Inch in 24 hours.	1 Inch in 24 hours.	1 Inch in 24 hours.
100 sup. feet	gallons	gallons per min.	gallons per min.	gallons per min.	gallons per min.	gallons per min.	gallons per min.	gallons per min.
52	0·14	0·87	0·43	0·22	0·11	0·036	0·018	0·005
104	0·28	1·74	0·87	0·43	0·22	0·072	0·036	0·009
156	0·43	2·60	1·30	0·65	0·32	0·108	0·054	0·013
208	0·57	3·47	1·74	0·87	0·43	0·144	0·072	0·018
260	0·71	4·34	2·17	1·08	0·54	0·181	0·090	0·022
1,000	520	1·4	8·7	4·3	2·2	1·1	0·36	0·09
2,000	1,040	2·8	17·4	8·7	4·3	2·2	0·72	0·18
3,000	1,560	4·3	26·0	13·0	6·5	3·2	1·08	0·27
4,000	2,080	5·7	34·7	17·4	8·7	4·3	1·44	0·72
5,000	2,600	7·1	43·4	21·7	10·8	5·4	1·81	0·90
10,000	5,200	14·2	86·8	43·4	21·7	10·8	3·62	1·81
1 acre	22,651	62	377	189	94	47	15·7	7·9
2 acres	45,302	124	755	377	189	94	31·5	15·7
3 "	67,954	186	1,132	566	284	142	47·2	23·6
4 "	90,605	248	1,510	755	378	189	63·0	31·5
5 "	113,256	310	1,887	944	472	236	78·7	39·3

QUANTITY and DISCHARGE from AREAS due to RAINFALL.

Area.	Quantity equal to 1 Inch of Rain over Surface.	Equivalent Supply Daily throughout the Year.	Quantity running off at following Rates.					
			1 Inch in an hour.	1/4 Inch in an hour.	1/8 Inch in an hour.	1/16 Inch in 24 hours.	1/32 Inch in 24 hours.	1/64 Inch in 24 hours.
10 acres	226,512	620	gallons	gallons per min.	gallons per min.	gallons per min.	gallons per min.	gallons per min.
20 "	453,025	1,241		3,775	1,888	944	472	157
30 "	679,537	1,862		7,550	3,775	1,888	944	315
40 "	906,049	2,482		11,326	5,663	2,831	1,415	472
50 "	1,132,561	3,103		15,101	7,550	3,776	1,888	629
				18,876	9,438	4,719	2,360	787
100 "	2,265,122	6,206		37,752	18,876	9,438	4,719	1,573
200 "	4,530,245	12,412		75,504	37,752	18,876	9,438	3,146
300 "	6,795,367	18,618		113,256	56,628	28,314	14,152	4,717
400 "	9,060,490	24,823		151,008	75,504	37,752	18,876	6,292
500 "	11,325,612	31,029		188,760	94,380	47,190	23,595	7,865
1 square mile	14,496,770	39,717		241,613	120,806	60,403	30,201	10,067
								5,033
								2,516
								1,258

E It is estimated that on an average four-fifths of the Rain runs off slated roofs, one-half off streets and paved surfaces; and one-eighth part off the surface of cultivated land, within an hour of falling, whenever the fall is considerable.

TABLE X.—ANNUAL RAINFALL.

Average Rainfall for 30 Years (1870–1899) in British Isles.

Division.	County.	Station.	Height above Sea.	Average Rainfall.
	ENGLAND.			
I.	Middlesex ..	London (Camden Square)	111	25·16
II.	Surrey	Reigate (Nutwood)	440	30·11
	Kent	Selling (Harefield)	217	29·55
	Sussex	Eastbourne (Osborne House) ..	12	30·98
	Hants	Osborne (Newbarn Cottage) ..	172	28·12
	"	Alton (Ashdell)	433	33·20
III.	Herts	Hitchin (Wratten)	238	24·66
	Bucks	High Wycombe	253	24·93
	Oxford	Oxford (Magdalen College) ..	186	24·54
	Northampton	Wellingboro (Croyland Abbey) ..	160	25·31
	Cambridge	Ely (Stretham)	42	22·16
IV.	Essex	Chelmsford (High Street)	86	22·96
	Suffolk	Ixworth (Walsham-le-Willows) ..	—	25·87
	Norfolk	Geldeston	38	23·93
	"	Hillington School	94	27·17
V.	Wilts	Marlborough (Mildenhall)	456	30·19
	Dorset	Wimborne Minster (Chisbury) ..	338	31·06
	Devon	Ashburton (Druid House) ..	572	52·91
	"	Barnstaple (Athenæum) ..	25	38·32
	Cornwall	St. Austell (Trevarna) ..	300	47·16
	Somerset	E. Harptree (Sherborne Reservoir)	338	41·16
VI.	Hereford	Ross (The Graig)	213	29·51
	"	Kington (Lynhales)	566	33·56
	Salop	Church Stretton (Woolstaston) ..	800	33·04
	"	Adderley Rectory	277	29·13
	Stafford	Burton (Rangemoor)	424	28·01
	Worcester	Northwick Park	410	29·22
VII.	Leicester	Thornton Reservoir	371	26·48
	Lincoln	Horncastle (Revesby)	135	24·77
	Notts	Worksop	56	24·54
VIII.	Cheshire	Woodhead Reservoir	660	48·85
	Lancashire	Ormskirk (Rufford)	39	33·71
	"	Cartmel (Holker)	155	43·69
IX.	York, W. Riding	South Milford Rectory	70	26·08
	" " "	Arncliffe Vicarage	734	60·96
	" E. "	Hull (Pearson Park)	6	27·02
	" N. "	Old Malton	75	26·71
	" " "	Bedale (Thorpe Perrow)	170	27·09

TABLE X.—*continued.*

Division.	County.	Station.	Height above Sea.	Average Rainfall.
			ft.	in.
	ENGLAND— <i>cont.</i>			
X.	Durham	Wolsingham	464	34·75
	Northumberland	Haltwhistle (Unthank Hall) ..	380	35·44
	"	Ilderton (Lilburn Tower) ..	300	29·19
	Cumberland	Whitehaven (Irish Street) ..	21	41·29
	"	Carlisle (Cemetery)	114	31·64
	Westmorland	Kendal (Ivy Garth)	146	50·41
	WALES.			
XI.	Pembroke	Haverfordwest (High Street) ..	95	47·88
	Carnarvon	Llanystumdwyr (Salarvor) ..	49	35·82
	"	Llandudno (Warwick House) ..	90	30·98
	SCOTLAND.			
XII.	Dumfries	Durrisdeer (Drumlanrig Castle)	191	44·28
XIII.	Selkirk	Galashiels (Abbotsford Road) ..	416	33·82
	Berwick	Marchmont House	500	34·91
XIV.	Lanark	Botwell Castle	146	28·92
	Ayr	Girvan (Pinmore)	187	48·87
	Renfrew	Waulk Glen	280	46·91
XVI.	Kinross	Loch Leven Sluice	360	36·20
	Perth	Loch Drunkie	420	63·09
	Forfar	Craigton	481	37·73
XVII.	Aberdeen	Braemar	1114	36·07
	Elgin or Moray	Gordon Castle	107	30·41
XVIII.	Inverness	Loch Shiel (Glenaladale)	50	105·29
XIX.	Sutherland	Golspie (Dunrobin Castle)	14	31·03
	IRELAND.			
XX.	Waterford	Portlaw (Mayfield)	70	42·38
XXI.	Wexford	Gorey (Courtown House)	80	35·72
	Wicklow	Bray (Fassaroe)	250	40·55
	Carlow	Carlow (Browne's Hill)	291	34·44
XXII.	Galway	Ballynasloe	160	37·04
XXIII.	Cavan	Belturbet (Red Hills)	208	35·19
	Armagh	Armagh Observatory	205	31·36
	Down	Seaford	180	38·61
	Tyrone	Omagh (Edenfel)	280	37·85

TABLE XI.—MONTHLY AND ANNUAL RAINFALL.

(1) Rainfall at Camden Square, London, during each Month for 42 Years, 1858–1899.

Year.	Jan.	Feb.	Mar.	April	May	June	July	Aug.	Sept.	Oct.	Nov.	Dec.	Total.
	in.	in.	in.	in.	in.	in.	in.	in.	in.	in.	in.	in.	in.
1858	.88	1.80	.69	2.90	2.76	.92	3.01	1.10	.85	1.58	.53	1.75	18.77
1859	.72	1.23	1.33	2.61	2.13	2.90	2.93	2.65	4.04	2.53	2.90	2.24	28.21
1860	1.97	1.25	1.87	1.45	3.57	5.47	2.26	4.48	2.92	1.77	2.72	2.51	32.24
1861	.43	1.93	2.43	1.30	1.39	2.13	2.42	.94	2.15	1.05	4.65	1.45	22.27
1862	1.92	.81	3.69	2.30	3.06	2.43	2.61	2.74	2.19	3.50	1.13	1.71	27.59
1863	2.80	.67	.85	.52	1.27	4.86	.92	1.44	3.49	1.62	1.84	1.31	21.59
1864	1.02	.85	2.62	.82	1.86	1.28	.62	1.33	2.55	1.13	2.49	.36	16.93
1865	3.90	2.01	1.12	.33	3.40	2.21	2.33	4.10	.55	6.22	1.96	1.35	29.48
1866	3.90	3.72	1.69	1.76	2.03	3.98	1.19	2.76	3.89	2.32	1.73	2.63	31.60
1867	2.81	1.44	2.48	2.36	2.45	1.22	4.30	2.63	2.23	1.92	.86	1.59	26.29
1868	3.89	1.21	1.28	1.50	1.58	.78	.45	2.28	1.74	2.54	1.03	5.12	23.40
1869	2.76	2.48	1.97	1.28	3.27	1.03	.62	1.26	3.56	1.87	2.38	2.94	25.42
1870	1.38	1.21	2.31	.47	.70	.83	1.22	2.69	2.00	3.68	1.76	3.07	21.32
1871	1.99	1.27	1.19	2.84	.92	3.49	4.12	.85	5.28	1.34	.60	1.13	25.02
1872	3.46	.96	2.66	1.39	3.05	2.55	2.57	2.05	1.64	5.20	3.98	4.35	33.86
1873	2.44	1.96	1.46	.55	1.56	2.24	2.81	2.87	2.46	2.97	1.87	.48	22.67
1874	1.18	.91	.39	1.26	1.14	2.05	.82	1.32	2.62	3.34	2.21	1.58	18.82
1875	3.22	1.06	.69	1.53	1.61	2.40	4.63	1.79	2.86	4.35	3.36	.94	28.44
1876	.94	1.97	2.96	1.90	.94	1.27	.81	1.79	2.86	1.40	3.07	6.25	26.16
1877	4.74	1.78	2.38	2.59	1.91	.42	3.94	2.23	.82	1.97	3.88	1.51	28.17
1878	1.31	1.49	1.12	4.97	3.89	6.71	.64	6.72	.83	1.99	2.95	1.46	34.08
1879	2.87	3.77	.91	2.72	3.46	4.76	4.17	5.11	3.67	.80	.72	.86	33.82
1880	.31	2.33	.79	2.15	.26	4.04	5.11	.45	4.04	5.78	1.85	3.17	30.28
1881	1.85	3.09	2.30	.46	1.52	1.72	1.85	4.89	2.03	2.99	2.75	2.47	27.92
1882	1.30	1.30	1.35	2.83	1.20	2.30	2.95	1.48	2.39	4.96	2.57	2.51	27.14
1883	2.08	3.62	.86	1.56	1.97	1.35	2.92	.93	3.83	1.75	2.78	.75	24.40
1884	2.30	1.40	1.41	1.02	.78	2.84	2.46	.89	1.77	.99	1.92	2.57	20.35
1885	1.43	2.86	1.65	2.32	2.63	1.99	.52	.85	4.30	3.73	3.31	1.05	26.64

TABLE XI.—*continued.*

Year.	Jan.	Feb.	Mar.	April	May	Junes	Jnly	Aug.	Sept.	Oct.	Nov.	Dec.	Total.
	in.	in.	in.	in.	in.	in.	in.	in.	in.	in.	in.	in.	in.
1886	4·02	·63	1·38	1·22	4·79	·63	2·37	·76	1·73	2·43	2·71	4·34	27·01
1887	1·26	·48	1·65	1·41	1·45	·91	1·07	3·15	1·81	1·24	3·40	1·38	19·21
1888	·90	·78	3·34	2·37	1·18	2·31	4·91	3·61	1·43	1·23	4·38	1·29	27·73
1889	·81	2·28	1·36	2·06	3·22	2·03	2·64	1·80	1·77	3·75	·89	1·23	23·84
1890	2·46	1·04	1·76	2·02	1·25	2·82	4·19	1·55	·64	1·20	1·62	·68	21·23
1891	1·80	·01	2·01	1·13	2·72	·86	3·82	4·75	1·03	4·80	1·98	3·24	28·15
1892	·50	1·62	1·04	·99	1·51	2·46	1·62	3·06	2·12	3·78	2·53	1·37	22·60
1893	1·44	2·87	·32	·24	·80	·73	2·46	1·61	1·07	3·87	2·16	2·23	19·80
1894	2·87	1·74	1·18	1·74	1·85	1·84	3·25	2·85	1·04	4·45	2·85	2·28	27·94
1895	1·96	·12	1·42	1·34	·34	·30	3·42	3·09	1·28	2·84	3·17	2·19	21·47
1896	·78	·29	3·20	·55	·14	2·27	1·03	1·92	5·51	3·05	1·17	3·61	23·52
1897	2·05	2·75	3·42	1·57	1·08	1·87	·64	2·92	2·75	·56	1·05	2·20	22·86
1898	·73	1·08	1·46	1·01	2·26	1·11	1·09	1·18	·33	2·96	1·94	2·54	17·69*
1899	2·52	2·00	·50	2·64	1·38	1·49	1·45	·70	2·65	2·03	4·13	1·05	22·54
Mean	2·00	1·58	1·68	1·67	1·91	2·19	2·33	2·31	2·35	2·70	2·33	2·12	25·20

Greatest fall in one civil year (1878), 34·08.

" " twelve months (March 1878 to February 1879), 37·92.

" " six months (March to August 1878) 24·06.

" " three months (March, April, May 1878), 15·57.

" " two months (December 1876, January 1877), 10·99

" " one month (August 1878), 6·72.

Least fall in one civil year (1864), 16·93.

" " twelve months (October 1897 to September 1898), 14·06.

" " six months (December 1873 to June 1874), 6·36.

" " four months (December 1873 to March 1874), 2·96.

" " three months (February, March, April, 1863), 1·94.

" " two months (March, April, 1893), ·56.

" " one month (February 1891), ·01.

Least average of three consecutive years (1897-8-9), 21·03.

* This was the total fall registered at Camden Square, but much lower records were obtained at other stations at lower elevation, viz. at Shoreditch, 14·30; East Ham, 14·08; Barking Outfall, 13·04 thus making 1898 the driest year for half a century over a considerable area.

TABLE XI.—*continued.*

(2) Average Monthly Rainfall at various stations in British Isles during 30 Years, 1870–1899.

Station.	Jan.	Feb.	March	April	May	June	July	Aug.	Sept.	Oct.	Nov.	Dec.	Total.
ENGLAND.													
Camden Square .	1·9	1·6	1·6	1·7	1·7	2·1	2·5	2·4	2·3	2·8	2·4	2·1	25·0
Eastbourne .	2·7	2·1	2·0	1·9	1·6	1·8	2·4	2·4	3·1	4·1	3·7	3·2	31·0
Hitchin .	1·8	1·5	1·5	1·6	1·9	1·9	2·5	2·3	2·3	2·7	2·6	2·0	24·7
High Wycombe .	2·2	1·8	1·6	1·6	1·6	1·8	2·1	2·1	2·4	2·9	2·6	2·3	24·9
Ely .	1·3	1·1	1·1	1·4	1·8	2·0	2·8	2·4	2·2	2·4	2·1	1·5	22·2
Marlborough .	2·6	2·2	1·9	2·0	1·9	2·2	2·8	2·7	2·6	3·3	3·3	2·7	30·2
Barnstaple .	3·4	2·8	2·3	2·2	2·1	2·3	3·3	3·4	3·6	4·9	4·0	4·1	38·5
Ross (Hereford)	2·7	2·2	1·7	1·9	2·1	2·3	2·8	2·6	2·7	3·1	2·9	2·4	29·4
Ormskirk .	2·7	2·0	2·2	1·7	2·1	2·4	3·4	3·6	3·4	3·9	3·2	3·1	33·7
Cartmel (Lancs.)	3·9	2·9	3·1	2·2	2·4	2·8	3·9	4·4	4·4	5·0	4·3	4·2	43·7
Old Malton (Yorks)	1·9	1·7	1·8	1·7	1·9	2·1	2·6	2·7	2·3	3·1	2·5	2·4	26·7
Kendal .	5·2	3·7	3·8	2·4	2·9	3·0	4·3	4·9	4·6	5·3	4·9	5·3	50·4
WALES.													
Haverfordwest .	5·1	3·7	3·0	2·6	2·5	2·6	3·7	4·0	4·2	5·6	5·4	5·2	48·0
Llandudno .	2·6	2·0	2·0	1·8	1·8	2·0	2·6	2·9	2·9	4·1	3·4	2·9	31·1
SCOTLAND.													
Bothwell Castle (Lanark) .	2·6	1·9	1·9	1·4	1·9	2·2	2·9	3·2	2·7	2·6	2·8	2·8	28·9
Waulk Glen (Renfrew) .	5·2	3·7	3·5	2·3	2·7	2·9	3·4	4·3	4·3	4·6	5·0	5·2	46·9
Loch Leven .	3·3	2·8	2·6	2·0	2·3	2·4	3·1	3·7	2·9	3·6	3·7	3·7	36·2
Craigton .	3·0	2·9	2·6	2·6	2·5	2·7	3·6	4·1	3·2	3·5	3·5	3·5	37·7
Braemar .	2·9	2·7	2·4	2·2	2·4	2·4	2·9	3·8	3·2	4·1	3·9	3·1	36·0
IRELAND.													
Portlaw (Waterford) .	4·5	3·7	2·7	2·9	2·5	2·6	3·2	3·9	3·2	4·3	4·1	4·7	42·2
Bray .	3·8	3·6	2·9	2·8	2·6	2·5	2·9	3·3	3·0	4·7	4·4	3·9	40·5
Ballinasloe .	3·5	2·5	2·4	2·4	2·5	2·7	3·4	3·9	3·2	3·6	3·6	3·6	37·0
Armagh .	2·6	2·1	2·0	2·0	2·1	2·5	3·2	3·3	2·9	3·0	2·8	2·8	31·3
Omagu (Tyrone) .	3·4	2·5	2·5	2·2	2·4	2·9	3·3	4·0	3·6	3·7	3·5	3·8	37·8
Average of 24 Stations .	3·1	2·5	2·3	2·1	2·2	2·4	3·1	3·3	3·1	3·7	3·5	3·4	34·8

TABLE XII.—DAILY and HOURLY MAXIMUM RAINFALL.

Period.	Greatest Ordinary Heavy Fall (as defined in "British Rainfall," all beyond this being recorded as "Exceptional").	Exceptional Falls recorded during the Years 1870 to 1899.
hours		Fall during the Year.
	2½ inches, where the total fall during the year exceeds 33 inches.	8·03 at Seathwaite, Cumberland, in 1897 143·4
		7·74 at Ben Nevis Observatory in 1894 151·7
		6·70 at Angerton, near Morpeth, in 1898 36·9 (During an extraordinary storm which lasted only about 3 hours.)
		6·00 at Tongue, Sutherland, in 1870 35·1
		5·00 at Blaenau Festiniog, in 1898 126·9
24	7·5 per cent. of the fall during the year, where it does not exceed 33 inches.	4·78 at Sittingbourne, being 17·7 p. c. of 27·0. 4·48 at Fakenham, being 16·2 p. c. of 27·6. 4·45 at N. Ockendon, Essex, being 16·5 p. c. of 27·0. 4·83 at Churchstoke, Montgomery, being 16·1 p. c. of 30. 4·93 at Galway, being 13 p. c. of 37·9.
2	{ 1 inch, or at rate of .50 in. per hr.}	3·75 inches. Flax Bourton, Somerset, July 16, 1892. 3 inches. Rotherham, September 15, 1880.
1½	{ .85 inch, or at rate of .56 in. per hr.}	3·07 inches = 2·05 in. per hour. Athlone, June 25, 1880.
1	.75 inch	2·58 inches. Sale, July 25, 1886.
min.		
45	{ .65 inch, or at rate of .87 in. per hr.}	
30	{ .50 inch, or at rate of 1 in. per hr.}	2·90 inches = 5·80 in. per hour. Cowbridge, South Wales, July 22, 1880.
20	{ .40 inch, or at rate of 1·20 in. per hr.}	1·48 inches = 4·44 in. per hour. Barnstaple, June 30, 1879.
15	{ .35 inch, or at rate of 1·40 in. per hr.}	0·75 inch = 3 in. per hour. Oxford, August 6, 1898.
10	{ .30 inch, or at rate of 1·80 in. per hr.}	1 inch = 6 in. per hour. London, June 23, 1878.
5	{ .20 inch, or at rate of 2·40 in. per hr.}	.40 inch in 3 minutes = 8 in. per hour. London, June 23, 1878.

TABLE XIII.—WATER SUPPLY by GRAVITATION—
NOTE.—Dimensions of Service Reservoirs and Distributing

Population.	Supply Required at 20 Gallons per Head.		Area of Gathering Ground for 12 Inches Available Rainfall.	Storage Reservoir to Hold Supply for 150 Days.			
	Daily.	Equiva- lent per Minute.					
	gallons	gallons	acres				
500	10,000	7	13½	175 ft. diam. by 10 ft. deep			
1,000	20,000	14	27	226	„	12	„
2,000	40,000	28	53½	320	„	12	„
3,000	60,000	42	80½	{ 391 2½ acres by	12	12	„
5,000	100,000	70	134		„	15	„
6,000	120,000	84	161	4½	„	15	„
8,000	160,000	112	215	6	„	15	„
10,000	200,000	139	268	{ 7½ 5½	„	15	„
20,000	400,000	278	536		„	20	„
30,000	600,000	417	805	15	„	15	„
50,000	1,000,000	694	1340	11	„	20	„
60,000	1,200,000	833	1610	16½	„	20	„
80,000	1,600,000	1,111	2145	44	„	20	„
100,000	2,000,000	1,389	sq. miles	{ 55 44	„	20	„
500,000	10,000,000	6,944			„	25	„
1,000,000	20,000,000	13,889		{ 220 183	„	25	„
				{ 440 367	„	30	„

WORKS for GIVEN POPULATION.

Mains same as for Pumping Works. (See next page.)

Filter Beds to Pass 600 Gallons per Super. Yard in 24 Hours, allowing for one not in use.

Main Conduit to Pass Supply in 24 Hours, flowing continuously.

No. 2, each 15 ft. by 10 ft.

" " 20 " 15 "

No. 3, " 30 " 10 "

" " 30 " 15 "

" " 50 " 15 "

" " 50 " 18 "

" " 60 " 20 "

No. 4, " 50 " 20 "
or 32 ft. sq.

No. 4, each 45 ft. square ..

" " 55 " ..

" " 70 " ..

" " 76 " ..

" " 90 " ..

No. 6, " 77½ " ..

" " 173 " ..

" " 245 " ..

{ 1½ inch, loss of head 1 in 120

2 " " 1 " 400

3 " " 1 " 120

1 " 1000

3 " " 1 " 240

4 " " 1 " 1000

4 " " 1 " 450

5 " " 1 " 1200

6 " " 1 " 160

1 " 1200

5 " " 1 " 350

6 " " 1 " 900

6 " " 1 " 500

7 " " 1 " 1000

6 " " 1 " 300

8 " " 1 " 1250

9 " " 1 " 600

10 " " 1 " 1000

10 " " 1 " 450

12 " " 1 " 1000

12 " " 1 " 400

15 " " 1 " 1200

12 " " 1 " 275

15 " " 1 " 850

15 " " 1 " 480

18 " " 1 " 1200

18 " " 1 " 750

21 " " 1 " 1700

{ 2½ feet, " 1 " 400

3 " " 1 " 1000

3 " " 1 " 250

4 " " 1 " 1000

TABLE XIV.—WATER SUPPLY by PUMPING—

Population.	Supply Required at 20 Gallons per Head.		Hours during which it is proposed to Pump.	Net Horse-power to raise to 100 Feet Elevation.
	Daily.	Equivalent per Minute.		
500	gallons 10,000	gallons 7	4	1½
1,000	20,000	14	6	1¾
2,000	40,000	28	10	2
3,000	60,000	42	10	3
5,000	100,000	70	10	5
6,000	120,000	84	10	6
8,000	160,000	112	10	8
10,000	200,000	139	10	10½
20,000	400,000	278	18	11¼
30,000	600,000	417	24	12½
50,000	1,000,000	694	24	21
60,000	1,200,000	833	24	25½
80,000	1,600,000	1,111	24	33½
100,000	2,000,000	1,389	24	42
500,000	10,000,000	6,944	24	210
1,000,000	20,000,000	13,889	24	421

WORKS for GIVEN POPULATION.

Dimensions of Single Pump, working 10 Strokes per Minute.			Dimensions of Pumping Main.		Service Reservoir to hold Three Days' Supply.		Main Delivery Pipe to Pass at Rate of One-half in Four Hours.	
Diam.	Stroke.		Diam.	Loss of Head.			Diam.	Loss of Head.
in. 8	ft. 2	in. 0	in. 3	1 in 110	22 ft. sq. by 10 ft. deep		in. 3	1 in 400
9	2	0	4	1 „ 450	31 „ 10 „		4	1 „ 450
10	2	0	5	1 „ 500	40 „ 12 „		5	1 „ 350
12	2	1	5	1 „ 240	49 „ 12 „		6	1 „ 380
14	2	6	6	1 „ 220	56½ „ 15 „		8	1 „ 580
15	2	8	7	1 „ 330	62 „ 15 „		8	1 „ 400
16	3	0	8	1 „ 350	71½ „ 15 „		9	1 „ 400
18	3	1	9	1 „ 400	80 „ 15 „		10	1 „ 450
18	3	4½	9	1 „ 335	98 „ 20 „		15	1 „ 850
18	3	9	10	1 „ 450	120 „ 20 „		15	1 „ 440
21	5	0	12	1 „ 400	155 „ 20 „		18	1 „ 310
24	4	3	15	1 „ 850	170 „ 20 „		21	1 „ 500
24	5	8	15	1 „ 475	196 „ 20 „		24	1 „ 570
24	7	0	18	1 „ 770	220 „ 20 „		27	1 „ 650
3·9	10	0	ft. 2 in. 6	1 „ 385	438 „ 25 „	ft. 4 in. 0	1 „ 500	
5·0	11	4	3 0	1 „ 245	620 „ 25 „	6 0	1 „ 880	

TABLE XV.—ANALYSIS OF WATER.

The Results are given in parts per 100,000. To convert into grains per gallon (the measure adopted by many analysts for some of the constituents) multiply by seven-tenths. Grains per gallon of Hardness are generally described as "degrees of hardness."

Source or Description.	Total Solids in Solution.	Hardness. Total. Parts per million.	Nitrogen as Nitrates.	Chlorine Parts per million.	Oxygen absorbed in 4 hours.	Ammonia. Free.	Albu- minoid.	Remarks.
<i>Waters supplied by London Companies.</i>								
New River (River Lea and Wells) ..	29.3	19.2	6.2	230	1.84	.059	.0009	.0049
East Loudon (River Lea) ..	29.0	19.1	6.2	209	1.99	.091	.0013	.0008
West Middlesex (Thames) ..	29.9	18.7	6.2	214	1.79	.109	.0009	.007
Southwark and Vauxhall (Thames) ..	28.8	18.8	6.3	251	1.80	.099	.0009	.0086
Grand Junction (Thames) ..	29.9	18.6	6.6	218	1.81	.102	.0010	.0074
Lambeth (Thames) ..	28.8	18.8	6.3	250	1.86	.106	.0006	.007
Chelsea (Thames) ..	29.1	18.5	6.5	218	1.81	.089	.0012	.0065
<i>Water supplied from deep wells.</i>								
Chalk—Kent (London Company) ..	33.2	22.2	7.8	334	2.39	.023	.0008	.0018
Canterbury ..	34.0	26.6	3.7	54	1.86	.018	.001	.001
Sudbury, Suffolk ..	53.0	28.4	2.4	43	4.8	.007	0	.002
Chalk, etc. (see Remarks)—Southend ..	96.0	2.8	..	02830.49	.037	0	.0036	.0036
Artesian Well at Blackfriars ..	74.8	7.0	..	.02	13.67	.015	.015	.004
Artesian Well at Newington ..	123.2	16.7	..	0	16.0	.035	.093	.004
New Red Sandstone—Wolverhampton ..	27.0	16.7	9.2	.071	2.14	.004	0	0
Coventry (Whitley) ..	37.0	35.0	9.2	.67	2.00	..	.004	0
Liverpool (Green Lane Well) ..	32.8	26.8	..	.482	3.25	.002	.001	.007
Kentish Ring Stone, near Maidstone ..	46.9	24.8	9.3	.665	3.28	.025	.0005	.0015

These figures represent the averages of analyses taken weekly throughout the year 1892.

During floods on the River Thames at same period, the oxygen absorbed by waters of the Thames companies increased to .160, and the aluminoid ammonia to .014.

The borings are taken into the chalk, but the water is derived principally from the Reading Beds overlying same.

Oolites—Spalding	68·4	10·0	0	14·85	·056	·074	·004
Peterborough	40·5	28·2	6·7	0	1·95	..	·002
Keuper or Marl Beds, Burton-on-Trent ..	220·0	·08	6·5	..	·003
Carboniferous Limestone, Ingletton, Yorks	13·5	10·0	4·0	·005
<i>Waters from shallow wells.</i>							
Burnham, Essex (public supply)	49·3	21·4	11·4	1·20	5·0	·050	·0005
St. Neots, Hunts (public supply)	51·4	31·4	14·3	·98	3·4	·033	·003
Burton-on-Trent (private wells)	111·0	1·6	8·5	..	·006
Southminster, Essex (private wells)	123·0	33·0	..	2·4	1·6	·25	·017
<i>Waters supplied from upland surfaces.</i>							
Glasgow, Loch Katrine	2·76	1·4	..	·006	·57	..	0
Manchester, Longendale	17·0	10·0	10·0	·07	1·4	..	·005
Liverpool, Rivington Pike	9·2	5·6	..	0	1·5	·045	·003
Liverpool, Lake Vyrnwy	4·16	2·4	..	0	·9	·132	·002
Kettering	20·4	15·0	7·0	·043	1·51	·100	0
Plymouth	2·8	2·1	2·1	·014	1·14	·124	·0007
<i>Other waters, &c.</i>							
River Thames at Hampton	34·0	19·0	6·5	·199	1·75	·186	·007
London Sewage—Northern outfall	86·0	15·7	4·46	4·32
Southern outfall	129·7	35·4	5·27	4·23
Croydon Sewage—Effluent from Farm ..	46·0	88	3·25	1·13
Sutton Sewage—Crude	157·9	0	11·47	2·94	3·00
Effluent from Bacteria	97·8	3·43	8·53	0·83	0·34
Beds	3800	800	750	·03	2000	..	·005
Sea Water	·027

Average of many brewery wells.
(The soils contain sulphates of lime and magnesia.)

Well in gravel beds.
Well 16 feet deep, in river gravel.

Average of a great many wells, various depths, in gravel overlying Keuper beds.
Average of 40 wells in gravel, liable to pollution.

Moorland, Lower Silurian rocks.
Moorland, Millstone Grit.
Moorland.
Moorland, Silurian rocks.
Cultivated land, subsoil, Northampton sand.
Principally moorland, subsoil granite.

" " "

The analyses of sewage are exclusive of suspended matters.

TABLE XVI.—QUANTITY of BRICKWORK in CIRCULAR SEWERS,
CULVERTS, OR WELLS.

NOTE.—The quantity of earth displaced will be the sum of the contents and brickwork added together.

Internal Diameter.	Contents of One Lineal Yard.	Brickwork per Lineal Yard.		Internal Diameter.	Contents of One Lineal Yard.	Brickwork per Lineal Yard.	
		4½ Inches Thick.	9 Inches Thick.			9 Inches Thick.	14 Inches Thick.
ft. in.	cub. ft.	cub. ft.	cub. ft.	ft. in.	cub. ft.	cub. ft.	cub. ft.
1 6	5·3	6·6	15·9	6 0	84·8	47·7	75·6
1 9	7·2	7·5	17·7	6 6	99·5	51·2	80·8
2 0	9·4	8·4	19·4	7 0	115·5	54·8	85·1
2 3	11·9	9·3	21·2	7 6	132·5	58·3	91·5
2 6	14·7	10·1	23·0	8 0	150·8	61·8	96·8
2 9	17·8	11·0	24·7	8 6	170·2	65·4	102·1
3 0	21·2	11·9	26·5	9 0	190·9	68·9	107·4
3 3	24·9	12·7	28·3	9 6	212·6	72·4	112·7
3 6	28·9	13·7	30·0	10 0	235·6	76·0	118·0
3 9	33·1	14·6	31·8	11 0	285·1	83·1	128·5
4 0	37·6	15·5	33·6	12 0	339·3	90·0	139·1
4 6	47·7	17·2	37·1	13 0	398·2	97·2	149·8
5 0	58·9	19·0	40·6	14 0	461·8	104·2	160·35
5 6	71·3	20·7	44·2	15 0	530·1	111·3	171·0

TABLE XVII.—QUANTITY of BRICKWORK in EGG-SHAPED SEWERS.

Internal Dimensions.	Contents of One Lineal Yard.	Brickwork per Lineal Yard.		Internal Dimensions.	Contents of One Lineal Yard.	Brickw per Lineal	
		4½ In. Thick.	9 In. Thick.			4½ In. Thick.	9 In. Thick.
ft. in. ft. in.	cub. ft.	cub. ft.	cub. ft.	ft. in. ft. in.	cub. ft.	cub. ft.	cub. ft.
2 0×1 4	6·0	7·4	16·5	3 6×2 4	18·5	11·6	25·5
2 3×1 6	8·2	8·1	18·8	3 9×2 6	21·2	12·4	26·9
2 6×1 8	9·4	8·8	20·1	4 0×2 8	24·2	13·0	28·3
2 9×1 10	11·4	9·5	21·4	4 6×3 0	32·9	14·4	31·1
3 0×2 0	13·6	10·2	22·7	5 0×3 4	37·7	15·8	34·0
3 3×2 2	15·9	10·9	24·0	6 0×4 0	54·2	18·8	39·4

In egg-shaped sewers about one-seventh part of the brickwork forms the invert, three-sevenths the top, and three-sevenths the sides. The two former should generally be built with radiating bricks of the radius required in each case.

TABLE XVIII.—WEIGHT of CAST-IRON PIPES.

NOTE.—The weight includes proportion due to sockets, pipes of 2 and $2\frac{1}{2}$ inches diameter being in 6-feet lengths, pipes 3 to 12 inches inclusive in 9-feet lengths, and those of larger size in 12-feet lengths, exclusive of socket.

Internal Diameter of Pipe.	For Pressure not exceeding 150 Feet.			For Pressure not exceeding 300 Feet.			For Pressure not exceeding 500 Feet.		
	Thickness of Metal.	Weight per Yard.		Thickness of Metal.	Weight per Yard.		Thickness of Metal.	Weight per Yard.	
inches	inch	cwt. 0	qrs. 0	lbs. 24	inch	cwt. 0	qrs. 0	lbs. 26	inch
2	$\frac{9}{32}$				$\frac{5}{16}$				$\frac{11}{32}$
$2\frac{1}{2}$	$\frac{5}{16}$	0	1	0	$\frac{11}{32}$	0	1	2	$\frac{3}{8}$
3	$\frac{5}{16}$	0	1	5	$\frac{11}{32}$	0	1	9	$\frac{5}{8}$
4	$\frac{11}{32}$	0	1	22	$\frac{5}{8}$	0	1	26	$\frac{7}{16}$
5	$\frac{3}{8}$	0	2	14	$\frac{7}{16}$	0	2	21	$\frac{1}{2}$
6	$\frac{3}{8}$	0	2	21	$\frac{7}{16}$	0	3	5	$\frac{1}{2}$
7	$\frac{7}{16}$	0	3	24	$\frac{1}{2}$	1	0	12	$\frac{9}{16}$
8	$\frac{7}{16}$	1	0	12	$\frac{1}{2}$	1	1	0	$\frac{9}{16}$
9	$\frac{1}{2}$	1	1	12	$\frac{9}{16}$	1	2	2	$\frac{5}{8}$
10	$\frac{1}{2}$	1	2	0	$\frac{9}{16}$	1	2	21	$\frac{5}{8}$
12	$\frac{9}{16}$	2	0	0	$\frac{5}{8}$	2	0	25	$\frac{11}{16}$
14	$\frac{5}{8}$	2	2	18	$\frac{11}{16}$	2	3	21	$\frac{3}{4}$
15	$\frac{5}{8}$	2	3	7	$\frac{11}{16}$	3	0	10	$\frac{13}{16}$
16	$\frac{5}{8}$	3	0	0	$\frac{3}{4}$	3	2	9	$\frac{7}{8}$
18	$\frac{11}{16}$	3	2	0	$\frac{3}{4}$	4	0	0	$\frac{15}{16}$
21	$\frac{11}{16}$	4	1	0	$\frac{13}{16}$	5	0	0	1
24	$\frac{3}{4}$	5	1	0	$\frac{7}{8}$	6	1	0	$1\frac{1}{8}$
27	$\frac{3}{4}$	6	0	0	$\frac{15}{16}$	7	2	0	$1\frac{3}{16}$
30	$\frac{7}{8}$	7	3	14	1	8	3	21	$1\frac{1}{4}$
36	1	10	2	21	$1\frac{1}{8}$	11	2	14	$1\frac{1}{2}$

TABLE XIX.—WEIGHT OF LEAD PIPES.

Note.—Columns 1, 2, and 3 are the pipes usually known as "common," "middling," and "strong" respectively, the figures in parenthesis show the weights per length of the coil according to which they are generally specified. The "common" are available only for pipes with open ends, the "middling" for very slight pressures, and the "strong" for pressure of about 50 feet.

Column 4 are the weights prescribed by the Metropolis Water Act, 1871, and by the regulations of very many towns, and are available for pressures up to 200 feet or thereabouts.

Column 5 are those prescribed at Norwich and some other towns where the pressure is unusually great.

Internal Diameter of Pipe.	Weight per Yard in Lbs.				
	No. 1.	No. 2.	No. 3.	No. 4.	No. 5.
$\frac{3}{8}$ inch	" "	" "	" "	5	$5\frac{1}{2}$
$\frac{1}{2}$ "	$3\frac{1}{3}$ (16 lbs. to 15 ft.)	$4\frac{2}{3}$ (22 lbs. to 15 ft.)	$5\frac{1}{3}$ (26 lbs. to 15 ft.)	6	7
$\frac{5}{8}$ "	" "	" "	" "	$7\frac{1}{2}$	9
$\frac{3}{4}$ "	$4\frac{1}{3}$ (24 lbs. to 15 ft.)	$5\frac{3}{5}$ (28 lbs. to 15 ft.)	$7\frac{1}{3}$ (36 lbs. to 15 ft.)	9	11
$\frac{1}{2}$ "	6 (30 lbs. to 15 ft.)	8 (40 lbs. to 15 ft.)	$9\frac{3}{5}$ (46 lbs. to 15 ft.)	12	16
$1\frac{1}{4}$ "	9 (36 lbs. to 12 ft.)	11 (44 lbs. to 12 ft.)	13 (53 lbs. to 12 ft.)	16	$22\frac{1}{2}$
$1\frac{1}{2}$ "	12 (48 lbs. to 12 ft.)	14 (56 lbs. to 12 ft.)	$17\frac{1}{2}$ (70 lbs. to 12 ft.)	24	33

